EXHIBIT A-16

PRELIMINARY STORMWATER MANAGEMENT REPORT

for

STOKES ESTATE

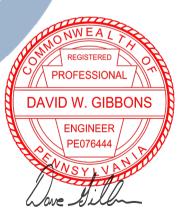
Residential Development Westtown Township Chester County, Pennsylvania

> April 29, 2021 Revised August 31, 2021 Revised September 29, 2021

> > D.L. Howell Job# 3868

Prepared for:

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D.L. HOWELL & ASSOCIATES, INC.

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1.0 INTRODUCTION

This Stormwater Management Report presents the preliminary permanent control measures/facilities required to support construction activities for the Stokes Estate Residential Development . The 65 +/-acre tract is located in Westtown Township (Figure 1-1).

The proposed land development consists of constructing 68 residential dwelling units, access roads, stormwater management facilities, and public utilities. One access point to the parcel will be off Shiloh Road, with a second access through an existing right of way to Shiloh Hill Drive. The buildings and roads will be constructed to comply with design standards and safety requirements of the Townships and local Fire Marshals.

1.1 LAND USE

The existing land is currently utilized for agriculture, with a few hedgerows and mature trees scattered throughout, mostly along the existing driveway and near the existing residence and outbuildings. The site generally drains to two separate water bodies, where the southern portion of the property drains to an Unnamed Tributary to the East Branch of Chester Creek that flows through the property, and the eastern end of the site drains directly to the East Branch of Chester Creek, also on the property. Therefore, the entire site is located in the Chester Creek watershed. Per Pennsylvania Department of Environmental Protection, 25 Pa. Code, 93.9g "Water Quality Standards" Chester Creek is classified as Trout Stocking Fishery (TSF).

1.2 SITE SOILS

Site soils mapping provided by the United States Department of Agriculture Natural Resources Conservation Service – Web Soil Survey. According to the Web Soil Survey mapping, the following soil types are located within the project study area;

Baile Silt Loam (Ba) Codorus Silt Loam (Co) Gladstone Gravelly Loam (GdB) (GdC) (GfD) Hatboro Silt Loam (Ha) Manor Loam (MaD) Urban land – Gladstone complex, 0 to 8 percent

Refer to Appendix E for Soils Map and report.

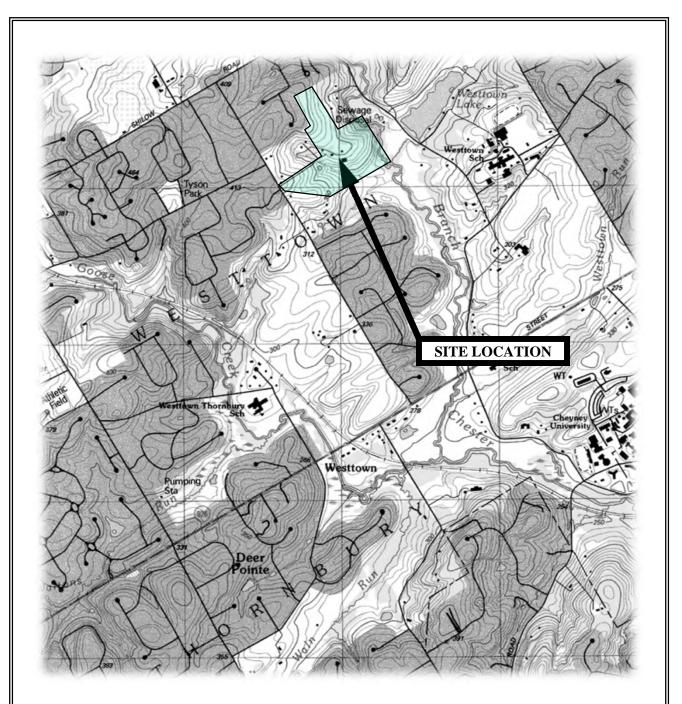
1.3 SOIL/GEOLOGIC LIMITATIONS:

Some groundwater and rock were encountered in a few locations during infiltration testing. D.L. Howell has taken into consideration these known soil limitations when designing the infiltration BMPs for the project. The stormwater infiltration facilities have either been relocated to areas where limiting areas weren't encountered or has been set a minimum of 2 feet higher than any prohibitive soil limitation elevations witnessed during infiltration testing and adequate infiltration results have been achieved at the adjusted elevations.

If during construction, any other unknown soil limitation (i.e. bedrock or high water) is discovered the contractor is responsible for immediately contacting the site geo-technical engineer, design engineer, conservation district and the township engineer for an appropriate solution. The site design drawings contain a pumped water filter bag detail which should be utilized if any excavations need to be dewatered due to high groundwater or excessive rainfall.

<u>Geologic formations/soil conditions that may have the potential to cause pollution:</u>

Furthermore, there are no known geologic formations or soil conditions that have the potential to cause pollution during earth disturbance activities. If during construction, an unknown geologic formations or soil conditions is discovered the contractor is responsible for immediately contacting the Chester County Conservation District and the design engineer.



Source:

United States Department of the Interior Geological Survey 7.5 Minute Series (Topographic) Map West Chester, Pennsylvania Quadrangle Scale 1:24000



Stokes Estate

Westtown Township Chester County, Pennsylvania Figure Number: FIGURE 1-1

Title: SITE LOCATION MAP

2.0 RUNOFF MANAGEMENT

The purpose of the stormwater management design is to quantify and control stormwater runoff generated by the modifications of the ground surface conditions to the site (i.e. roads, buildings, driveways, etc.). Post-development stormwater management is achieved at the site through three combination surface infiltration basins strategically located throughout the site to control runoff.

The infiltration basins have been designed utilizing Soil Conservation Service (SCS) method for infiltration and peak flow requirements and Westttown Township regulations for peak flow calculations (See Appendices for worksheets). The stormwater management control for this project was designed to include all impervious surfaces associated with this subdivision application, with an assumption of 3,800 SF of impervious coverage per single family lot. These systems are designed to provide an overall reduction in the post-developed runoff for the 2-year, 10-year, 25-year, 50-year, and 100-year, 24-hour storm event to less than 50% of the pre-development runoff rates for the equivalent storm events based on the Chester Creek Watershed Release Rate Map. A stormwater conveyance system will be utilized to convey runoff from the proposed improvements to the proposed stormwater facilities. The stormwater conveyance system will be designed to convey flows up to the 100-year storm event. Flows to the pipes will be generated using the Universal Rational Method and the pipes sized using Manning's Method and Hydraulic Grade Line calculations will also be provided. The infiltration basins have been designed and sized to fully infiltrate the increase in volume, pre to post-development for the 2-year storm as required by the NPDES Phase II regulations.

3.0 NPDES STORMWATER COMPLIANCE

As stated above, the infiltration facilities have been designed and sized to fully infiltrate the 2-year increase in volume; therefore the NPDES Phase II infiltration requirement has been met. Furthermore, as described above, the infiltration basins have been designed to incorporate Pennsylvania Department of Environmental Protection's infiltration guidelines, as stated in Appendix C of the Pennsylvania Stormwater Best Management Practices Manual dated December 2006. The stormwater management systems have been designed to maximize infiltration best management practice (BMP) technologies and minimize point source discharges. This plan will further act to perform/provide the following:

- Preserve the integrity of stream channels and maintain and protect the physical, biological and chemical qualities of the receiving stream by utilizing several BMPs to handle the increase in runoff and volume prior to reaching the stream.
- Prevent an increase in the rate of stormwater runoff by utilizing BMPs to reduce the peak flow rate of all storm events up to the 100 year to below the equivalent storm in the pre developed condition.
- Minimize any increase in stormwater runoff volume by utilizing infiltration BMPs which are designed and sized to fully infiltrate the 2-year increase in volume.
- Minimize impervious areas
- Maximize the protection of existing drainage features and existing vegetation by capturing stormwater runoff from the proposed impervious areas then conveying the flow to stormwater BMPs facilities prior to any release to the existing stream, thereby protecting it from any sediment.
- Minimize land clearing and grading by protecting and preserving the majority of the existing woodlands, and natural areas.
- Minimize soil compaction by specifying the installation of orange construction fencing to protect the areas of the proposed infiltration BMPs.
- Utilize other structural or nonstructural BMPs that prevent or minimize changes in stormwater runoff. The structural BMPs are infiltration beds, and water quality filters, while the non-

structural BMPs are protecting existing riparian buffers, minimizing total disturbed area, and protecting sensitive features.

D.L. Howell & Associates, Inc. has designed Best management Practices (BMP's) consistent with Chapter 6 of the PA Stormwater Best Management Practices Manual within the stormwater collection and conveyance system in addition to infiltrating the net increase in volume from pre to post-development for the 2-year storm event.

The applicant has been able to demonstrate compliance with 102.8(b), through the use of infiltration.

Permanent BMP's proposed for the developed site are as follows:

- Vegetated Swales
- Infiltration Basins
- Forebays
- Level Spreaders

4.0 CONCLUSIONS

D.L. Howell & Associates, Inc. has completed a preliminary stormwater engineering design for the proposed project in Westtown Township, Chester County, Pennsylvania. Using site-specific topography, soils, land cover, hydrologic data, and Township Ordinances, D.L. Howell & Associates, Inc. designed the stormwater management system for the proposed facilities. The objective of the stormwater design was to develop site-specific stormwater management structures that reduced post-development runoff to pre-development runoff rates and provided volumetric storage per PADEP NPDES Phase II requirements. Post-development stormwater management is achieved through a stormwater collection system consisting of curbed inlets, swales, catch basins, and stormwater infiltration basins/beds.

APPENDIX A

STORMWATER VOLUME CALCULATIONS



CHANGE IN RUNOFF VOLUME FOR 2-YR STORM EVENT

Worksheet 4, Pennsylvania Stormwater Best Management Practices Manual

PROJECT:	Stokes Estate			
Drainage Area:	DP001 Chester Creek			
2-Year Rainfall:	3.2 in			
Total Site Area	acres			

Total Sile Alea.		acres
Protected Site Area:		acres
Managed Area:	13.67	acres

Existing Conditions

Cover Type/Conditions	Soil Type	Area (sf)	Area (ac)	CN	s	la (0.2*S)	Q Runoff ¹ (in)	Runoff Volume ² (ft ³)
Woodland	Α		0.00	25	30.0000	6.0000	0.29	
Meadow	Α		0.00	30	23.3333	4.6667	0.10	
Impervious	Α		0.00	98	0.20	0.04	2.97	
Woodland	В		0.00	55	8.1818	1.6364	0.25	
Meadow	В	672,131	15.43	58	7.2414	1.4483	0.34	19,111
Meadow (20% Imperv)	В		0.00	58	7.2414	1.4483	0.34	
Impervious (80%)	В		0.00	98	0.2041	0.0408	2.97	
					-			
Woodland	С		0.00	70	4.2857	0.8571	0.83	
Meadow	С		0.00	71	4.0845	0.8169	0.88	
Impervious	С		0.00	98	0.2041	0.0408	2.97	
			•					
Woodland	D		0.00	77	2.9870	0.5974	1.21	
Meadow	D		0.00	78	2.8205	0.5641	1.27	
Impervious	D		0.00	98	0.2041	0.0408	2.97	
						·	• 	
TOTAL:		672,131	15.43					19,111

Developed Conditions

Cover Type/Conditions	Soil Type	Area (sf)	Area (ac)	CN	S	la (0.2*S)	Q Runoff ¹ (in)	Runoff Volume ² (ft ³)
Lawn	В	323,952	7.44	61	6.3934	1.2787	0.44	11,985
Impervious	N/A	153,121	3.52	98	0.2041	0.0408	2.97	37,865
Meadow	В	118,483	2.72	58	7.2414	1.4483	0.34	3,369
			0.00					
			0.00					
			0.00					
			0.00					
			0.00					
TOTAL:		595,556	13.67					53,220

2-Year Volume Increase (ft³):

34,108

2-Year Volume Increase = Developed Conditions Runoff Volume - Existing Conditions Runoff Volume

1. Runoff (in) = Q = (P - 0.2S)2 / (P + 08.S) P = 2-Year Rainfall (in) S = (1000/CN) - 10

2. Runoff Volume (CF) = Q x Area x 1/12

Q = Runoff (in) Area = Land Use Area (Sq. Ft)

Note: Runoff Volume must be calculated for EACH land use type/condition and HSGI. The use of a weighted CN value for volume calculations is not acceptable.



CHANGE IN RUNOFF VOLUME FOR 2-YR STORM EVENT

Worksheet 4, Pennsylvania Stormwater Best Management Practices Manual

PROJECT:	Rustin Residential				
Drainage Area:	DP002 UNT Chester Creek				
2-Year Rainfall:	3.2 in				

Total Site Area:		acres
Protected Site Area:		acres
Managed Area:	20.97	acres

Existing Conditions

Cover Type/Conditions	Soil Type	Area (sf)	Area (ac)	CN	S	la (0.2*S)	Q Runoff ¹ (in)	Runoff Volume ² (ft ³)
Woodland	Α		0.00	25	30.0000	6.0000	0.29	
Meadow	Α		0.00	30	23.3333	4.6667	0.10	
Impervious	Α		0.00	98	0.20	0.04	2.97	
Woodland	В		0.00	55	8.1818	1.6364	0.25	
Meadow	В	745,375	17.11	58	7.2414	1.4483	0.34	21,194
Meadow (20% Imperv)	В		0.00	58	7.2414	1.4483	0.34	
Impervious (80%)	В		0.00	98	0.2041	0.0408	2.97	
Woodland	С		0.00	70	4.2857	0.8571	0.83	
Meadow	С	91,390	2.10	71	4.0845	0.8169	0.88	6,687
Impervious	С		0.00	98	0.2041	0.0408	2.97	
·							•	
Woodland	D		0.00	77	2.9870	0.5974	1.21	
Meadow	D		0.00	78	2.8205	0.5641	1.27	
Impervious	D		0.00	98	0.2041	0.0408	2.97	
			·			·	•	
TOTAL:		836,765	19.21			1		27,882

Developed Conditions

Cover Type/Conditions	Soil Type	Area (sf)	Area (ac)	CN	S	la (0.2*S)	Q Runoff ¹ (in)	Runoff Volume ² (ft ³)
Lawn	С	75,360	1.73	74	3.5135	0.7027	1.04	6,516
Lawn	В	563,811	12.94	61	6.3934	1.2787	0.44	20,859
Impervious	N/A	274,153	6.29	98	0.2041	0.0408	2.97	67,795
			0.00					
			0.00					
			0.00					
			0.00					
			0.00					
			0.00					
TOTAL:		913,324	20.97					95,170

2-Year Volume Increase (ft³):

crease (ft³): 67,289

2-Year Volume Increase = Developed Conditions Runoff Volume - Existing Conditions Runoff Volume

1. Runoff (in) = Q = (P - 0.2S)2 / (P + 08.S) P = 2-Year Rainfall (in) S = (1000/CN) - 10

2. Runoff Volume (CF) = Q x Area x 1/12 Q = Runoff (in) Area = Land Use Area (Sq. Ft)

Note: Runoff Volume must be calculated for EACH land use type/condition and HSGI. The use of a weighted CN value for volume calculations is not acceptable.

Chapter 8



STRUCTURAL BMP VOLUME CREDITS

Worksheet 5, Pennsylvania Stormwater Best Management Practices Manual

PROJECT:	Stokes Estate Chester Creek DP001				
	Chlotter				
Required Control Volume	34,108	Cubic Feet			
Non-Structural Volume Credit	0	Cubic Feet			
Structure Volume Requirement	34,108	Cubic Feet			

Section	Proposed BMP	Area (sf)	Storage Volume (ft ³)	
6.4.1	Porous Pavement			
6.4.2	Infiltration Basin		35,889	
6.4.3	Infltration Bed			
6.4.4	Infiltration Trench			
6.4.5	Rain Garden/Bioretention			
6.4.6	Dry Well/Seepage Pit			
6.4.7	Constructed Filter			
6.4.8	Vegetated Swale			
6.4.9	Vegetated Filter Strip			
6.4.10	Infiltration Berm			
6.5.1	Vegetated Roof			
6.5.2	Capture and Re-Use			
6.6.1	Constructed Wetlands			
6.6.2	Wet Pond/Retention Basin			
6.6.3	Dry Extended Detention Basin			
6.6.4	Water Quality Filters			
6.7.1	Riparian Buffer Restoration			
6.7.2	Landscape Restoration / Reforestation			
6.7.3	Soil Amendment			
6.8.1	Level Spreader			
6.8.2	Special Storage Areas			
	Other			
		0	35,889	

Total Structural Volume (cf)	35,889
Structural Volume Requirement (cf)	34,108
DIFFERENCE	1,781

Chapter 8



STRUCTURAL BMP VOLUME CREDITS

PROJECT:

Sub-Basin

Worksheet 5, Pennsylvania Stormwater Best Management Practices Manual

Stokes Estate UNT Chester Creek DP002

Required Control Volume	67,289	Cubic Feet
Non-Structural Volume Credit	0	Cubic Feet
Structure Volume Requirement	67,289	Cubic Feet

Section	Proposed BMP	Area (sf)	Storage Volume (ft ³)	
6.4.2	Infiltration Basin 1 Combined		34,254	
6.4.2	Infiltration Basin 2 33,072			
6.4.3	Infltration Bed(s)			
6.4.5	Rain Garden/Bioretention			
6.4.6	Dry Well/Seepage Pit			
6.4.7	Constructed Filter			
6.4.8	Vegetated Swale			
6.4.9	Vegetated Filter Strip			
6.4.10	Infiltration Berm			
6.5.1	Vegetated Roof			
6.5.2	Capture and Re-Use			
6.6.1	Constructed Wetlands			
6.6.2	Wet Pond/Retention Basin			
6.6.3	Dry Extended Detention Basin			
6.6.4	Water Quality Filters			
6.7.1	Riparian Buffer Restoration			
6.7.2	Landscape Restoration / Reforestation			
6.7.3	Soil Amendment			
6.8.1	Level Spreader			
6.8.2	Special Storage Areas			
	Other			
		0	67,326	

Total Structural Volume (cf)	67,326
Structural Volume Requirement (cf)	67,289
DIFFERENCE	37

3930-PM-WM0035 / Rev 5/2007

Chapter 8



INFILTRATION VOLUME CALCULATION Basin 1 Upper

PROJECT NAME:	Stokes Estate		
LOCATION:	Westtown Township		
PREPARED BY:	DWG	DATE:	3/30/2021
CHECKED BY:	DLH	DATE:	

WATER SURFACE	AREA	AVERAGE	DIFFERENCE	STORAGE VOLU	ME (CUBIC FEET)
ELEVATION (FEET)	AREA (SQ.FT.)	AREA (SQ.FT.)	IN ELEVATION (FEET)	INCREMENTAL	TOTAL
320.00	13,236	(30.11.)			0
520.00	13,230	14,996	2.00	29991	0
322.00	16,755				29,991
		18,686	2.00	37372	
324.00	20,617				67,363
		22,559	2.00	45117	
326.00	24,500				112,480
		ļ			

	Proposed Infiltration Volur
	Storage Volum
Elevation	(CF)
322.00	29,991
<u>322.50</u>	<u>39,334</u>
324.00	67,363
Volume =	39,334 CF
volume =	39,334 CF

*Only 34,254 CF drains to the basin in the 2 year storm, therefore only 34,254 CF is being taken credit for



INFILTRATION VOLUME CALCULATION Basin 1 Lower

PROJECT NAME:	Stokes Estate		
LOCATION:	Westtown Township		
PREPARED BY:	DWG	DATE:	3/30/2021
CHECKED BY:	DLH	DATE:	

WATER SURFACE	AREA	AVERAGE	DIFFERENCE	STORAGE VOLU	ME (CUBIC FEET)
ELEVATION (FEET)	AREA (SQ.FT.)	AREA (SQ.FT.)	IN ELEVATION (FEET)	INCREMENTAL	TOTAL
302.00	2,215				0
		3,094	2.00	6187	
304.00	3,972				6,187
		7,254	2.00	14507	
306.00	10,535				20,694
		12,703	2.00	25405	
308.00	14,870				46,099

	Proposed Infiltratio	on Volume
Elevation 302.00 <u>304.00</u> 304.00	<u>.</u>	age Volume (CF) 0 <u>6,187</u> 6,187
Volume =	6,187 CF	



INFILTRATION VOLUME CALCULATION Basin 2

PROJECT NAME: S	Stokes Estate		
LOCATION: V	Vesttown Township		
PREPARED BY: D	DWG	DATE:	3/30/2021
CHECKED BY: D	DLH	DATE:	

WATER SURFACE	AREA	AVERAGE	DIFFERENCE	STORAGE VOLU	ME (CUBIC FEET)
ELEVATION	AREA	AREA	IN ELEVATION	INCREMENTAL	TOTAL
(FEET)	(SQ.FT.)	(SQ.FT.)	(FEET)		
306.00	8,255				0
		10,349	2.00	20698	
308.00	12,443				20,698
		14,558	2.00	29116	
310.00	16,673				49,814
		18,892	2.00	37783	
312.00	21,110				87,597

	Proposed Infiltration Volume
	Storage Volume
Elevation	(CF)
308.00	20,698
<u>306.85</u>	<u>3,956</u>
310.00	49,814
Volume =	3,956 CF
L	

\bigcirc	DLHowell
	Civil Engineering & Land Planning
	www.DLHowell.com

INFILTRATION VOLUME CALCULATION Basin 3

PROJECT NAME:	Stokes Estate		
LOCATION:	Westtown Township		
PREPARED BY:	DWG	DATE:	3/30/2021
CHECKED BY:	DLH	DATE:	

WATER SURFACE	AREA	AVERAGE	DIFFERENCE	STORAGE VOLU	ME (CUBIC FEET)
ELEVATION (FEET)	AREA (SQ.FT.)	AREA (SQ.FT.)	IN ELEVATION (FEET)	INCREMENTAL	TOTAL
316.00	15,345	(· · · · /		0
		17,505	2.00	35010	
318.00	19,665				35,010
		21,938	2.00	43876	
320.00	24,211				78,886
		25,391	1.00	25391	
321.00	26,570				104,277

	Proposed Infiltration	on Volum
	Storag	ge Volum
Elevation	Ī	(CF)
318.00	3	35,010
<u>318.50</u>	<u>4</u> :	5,97 <u>9</u>
320.00	78	78,886
r		
Volume =	45,979 CF	

*Only 35,889 CF drains to the basin in the 2 year storm, therefore only 35,889 CF is being taken credit for

APPENDIX B TOWNSHIP POST DEVELOPMENT FLOW REDUCTION SUMMARIES

	IOWEII ring & Land Planning rell com		vater Summary eduction Requiren		
					DATE: <u>9/30/2021</u> BY: <u>DWG</u>
JOB NO.: <u>3868</u> DESCRIPTION:		okes Estate v DP001 Chester Creek	TOWNSHIP: Westtown	<u>!</u>	
					% Reduction
	1-year	Pre-Developed	1.30 cfs	Hydrograph 1	
	1-year	Post-Developed	0.58 cfs	Hydrograph 7	55%
		•			
	2-year	Pre-Developed	4.32 cfs	Hydrograph 1	73%
	2-year	Post-Developed	1.15 cfs	Hydrograph 7	
	5-year	Pre-Developed	12.58 cfs	Hydrograph 1	
	5-year	Post-Developed	2.31 cfs	Hydrograph 7	82%
	0 900		2.01 0.0	ny di ographi i	
	10-year	Pre-Developed	20.75 cfs	Hydrograph 1	84%
	10-year	0-year Post-Developed 3.34 cfs Hydrograph 7		Hydrograph 7	04 %
	05	Dre Developed	00.00 -fr		
	25-year	Pre-Developed	33.98 cfs	Hydrograph 1	80%
	25-year	Post-Developed	6.86 cfs	Hydrograph 7	
	50-year	Pre-Developed	46.28 cfs	Hydrograph 1	
	50-year	Post-Developed	21.53 cfs	Hydrograph 7	53%
	· · · · · · · · · · · · · · · · · · ·		·		
	100-year	Pre-Developed	60.31 cfs	Hydrograph 1	52%
	100-year	Post-Developed	29.04 cfs	Hydrograph 7	02,0
			0 RELEASE RATE AREA d 2 Year Flow = 1.15	cfs	
			d 1 Year Flow = 1.30	cfs	SATISFIED
		Post Develope 50% Pre Develope	d 5 Year Flow = 2.31 d 5 Year Flow = 6.29	cfs cfs	SATISFIED
		Post Developed 50% Pre Developed		cfs cfs	SATISFIED
		Post Developed 50% Pre Developed		cfs cfs	SATISFIED
		Post Developed 50% Pre Developed		cfs cfs	SATISFIED
		Post Developed 7 50% Pre Developed 7		cfs cfs	SATISFIED

	owell ing & Land Planning		vater Summary eduction Requirem			
					DATE: BY:	<u>9/30/2021</u> <u>DWG</u>
JOB NO.: <u>3868</u> DESCRIPTION:		tokes Estate y DP002 UNT Chester Creek	TOWNSHIP: Westtown	L		
					% Reduction	
	1-year	Pre-Developed	2.47 cfs	Hydrograph 2		
	1-year	Post-Developed	0.42 cfs	Hydrograph 15	83%	
	2-year	Pre-Developed	6.85 cfs	Hydrograph 2		
	2-year	Post-Developed	0.96 cfs	Hydrograph 15	86%	
			40.00 (
	5-year	Pre-Developed	18.02 cfs	Hydrograph 2	88%	
	5-year	Post-Developed	2.12 cfs	Hydrograph 15		
	10-year	Pre-Developed	28.64 cfs	Hydrograph 2		
				Hydrograph 15	89%	
	25-year	Pre-Developed	45.70 cfs	Hydrograph 2		
	25-year	Post-Developed			86%	
	20 your		0.17 013	Hydrograph 15		
	50-year	Pre-Developed	46.28 cfs	Hydrograph 2	54%	
	50-year	Post-Developed	21.32 cfs	Hydrograph 15	5470	
	100-year	Pre-Developed	79.34 cfs	Hydrograph 2		
	100-year	Post-Developed	37.69 cfs	Hydrograph 15	52%	
			RELEASE RATE AREA			
			d 2 Year Flow = 0.96 d 1 Year Flow = 2.47	cfs cfs	SATISFIED	
				013		
		Post Develope 50% Pre Develope	d 5 Year Flow = 2.12 d 5 Year Flow = 9.01	cfs cfs	SATISFIED	
		Post Developed	10 Year Flow = 3.17	cfs		
		50% Pre Developed	10 Year Flow = 14.32	cfs	SATISFIED	
		Post Developed 50% Pre Developed		cfs cfs	SATISFIED	
		Post Developed 50% Pre Developed		cfs cfs	SATISFIED	
		Post Developed 1 50% Pre Developed 1		cfs cfs	SATISFIED	

APPENDIX C

SCS METHOD CURVE NUMBER (CN) CALCULATIONS

Civil Er	gineering	& Land Planning			HYDRO	OLOGIO	CONSERVATION C DATA FOR V RUNOFF COM	VATERSHED	
WWW.L	LHowell.co	om						DATE: BY:	<u>3/29/20</u> DW0
JOB NO.: DESCRIPTIC	<u>3868</u> DN:	PROJECT: PREDEVELOPED AR	Stokes Estate EA CHESTER CREI	EK DP001			TOWNSHIP:	<u>Westtown</u>	
Total Area:	15.43	acres							
Symbol	Soil Name	Hydrological Soil Group	Land Use	Hydrologic Condition	Soil Runoff Curve Number	Area acres	Complex Number acres	Comment	
GdB	Gladstone Loam	BB	Meadow Woods	Good Good	58 55	15.43 0.00	894.94 0.00		
Ва	Baile Silt Loam	C C	Meadow Woods	Good Good	71 70	0.00 0.00	0.00 0.00		
					Total Area	15.43	894.94		

Civil Er		Well Land Planning			HYDRO	OLOGIO	CONSERVATIO C DATA FOR V RUNOFF COM	VATERSHED	
VV VV VV.L	internet in the second se							DATE: BY:	<u>3/29/2</u> DW
JOB NO.: DESCRIPTI		PROJECT: PREDEVELOPED ARE	Stokes Estate EA UNT CHESTER (CREEK DP002			TOWNSHIP:	<u>Westtown</u>	
Total Area:	19.21 a	Hydrological	Land Use	Hydrologic	Soil Runoff	Area	Complex Number	Comment	
Symbol	Soil Name		Land Use	Condition	Curve Number	acres	acres		
		Soil Group							
GdB	Gladstone	B	Meadow	Good	58	17.11	992.38		
GdB									
GdB Ba	Gladstone	B B	Meadow	Good	58	17.11	992.38		
	Gladstone Loam	B	Meadow Woods	Good Good	58 55	17.11 0.00	992.38 0.00		
	Gladstone Loam Baile Silt	в В С	Meadow Woods Meadow	Good Good Good	58 55 71	17.11 0.00 2.10	992.38 0.00 149.10		
	Gladstone Loam Baile Silt Loam	в В С	Meadow Woods Meadow	Good Good Good	58 55 71 70	17.11 0.00 2.10 0.00	992.38 0.00 149.10 0.00		

Civil Er		Well & Land Planning			HYDR	OLOGIO	CONSERVATION C DATA FOR V RUNOFF COM	VATERSHED	
VV VV .L	Chowen.co							DATE: BY:	<u>3/29/2021</u> <u>DWG</u>
JOB NO.: DESCRIPTIO		POST DEVELOPED B	<u>Stokes Estate</u> ASIN 3				TOWNSHIP:	<u>Westtown</u>	
	.	Hydrological		Hydrologic	Soil Runoff	Area	Complex Number	Comment	l
Symbol	Soil Name	Soil Group	Land Use	Condition	Curve Number	acres	acres		
GdB	Gladstone	В	Meadow	Good	58	2.47	143.26		
	Loam	B	Lawn	Good	61	6.25	381.38		
Ва		N/A	Impervious Meadow	N/A Good	98 71	3.43	335.94 0.00		
Da	Baile Silt Loam	C C	Lawn	Good	74	0.00 0.00	0.00		
					Total Area	12.15	860.57		
Weighted Sc	bil	860.6	=	70.8					
Complex Nu		12.2			l				
2		· = · =			ASSUMES 5 MINUT		CONCENTRATION		

Civil Er		& Land Planning			HYDR	OLOGI	CONSERVATION C DATA FOR V RUNOFF COM	VATERSHED	
WWW.L	JEI IOWEII.C							DATE: BY:	<u>3/29/2021</u> <u>DWG</u>
JOB NO.: DESCRIPTI	<u>3868</u> ON:	PROJECT: POST DEVELOPED E	Stokes Estate 3YPASS DP002				TOWNSHIP:	<u>Westtown</u>	
Total Area:	1.49	acres							
Symbol	Soil Name	Hydrological Soil Group	Land Use	Hydrologic Condition	Soil Runoff Curve Number	Area acres	Complex Number acres	Comment	
GdB	Gladstone	B	Meadow	Good	58	0.00	0.00		
	Loam	B	Lawn	Good	61	1.40	85.40		
		N/A	Impervious	N/A	98	0.09	8.55		
Ва	Baile Silt	С	Meadow	Good	71	0.00	0.00		
	Loam	С	Lawn	Good	74	0.00	0.00		
					Total Area	1.49	93.95		
Weighted So	bil	93.9	=	63.2					
Complex Nu	mber	1.5	-						
					ASSUMES 5 MINUT	E TIME OF	CONCENTRATION		

0	DLHOWEII Civil Engineering & Land Planning www.DLHowell.com					SOIL CONSERVATION SERVIO HYDROLOGIC DATA FOR WATERSHE RUNOFF COMPUTATION				D
V	vww.DL	-nowell.co	m						DAT	TE: <u>3/29/2021</u> 3Y: <u>DWG</u>
DES	NO.: CRIPTIOI al Area:	<u></u>	POST DEVELOPED B	<u>Stokes Estate</u> ASIN 1				TOWNSHIP:	<u>Westtown</u>	
SV	mbol	Soil Name	Hydrological Soil Group	Land Use	Hydrologic Condition	Soil Runoff Curve Number	Area acres	Complex Number acres	Comment	
	GdB	Gladstone	B	Meadow	Good	58	0.00	0.00		
	Jub	Loam	В	Lawn	Good	61	7.57	461.77		
		Loam	N/A	Impervious	N/A	98	3.08	301.86		
	Ва	Baile Silt	C	Meadow	Good	71	0.00	0.00		
		Loam	C	Lawn	Good	74	0.30	22.20		
						Total Area	10.95	785.83		
-	ghted Soil		785.8	=	71.8					
Com	plex Num	ber	11.0			ASSUMES 5 MINUT	E TIME OF	CONCENTRATION		

Civil Er		Well & Land Planning			SOIL CONSERVATION SERVICE HYDROLOGIC DATA FOR WATERSHED RUNOFF COMPUTATIONS				
WWW.L	JEHOWEII.CO							DATE: BY:	<u>3/29/2021</u> <u>DWG</u>
JOB NO.: DESCRIPTI	<u></u>	POST DEVELOPED B	<u>Stokes Estate</u> ASIN 2				TOWNSHIP:	Westtown	
		Hydrological		Hydrologic	Soil Runoff	Area	Complex Number	Comment	
Symbol	Soil Name	Soil Group	Land Use	Condition	Curve Number	acres	acres		
GdB	Gladstone	В	Meadow	Good	58	0.00	0.00		
	Loam	В	Lawn	Good	61	4.41	269.01		
_		N/A	Impervious	N/A	98	3.23	316.23		
Ba	Baile Silt	С	Meadow	Good	71	0.00	0.00		
	Loam	C	Lawn	Good	74	0.90	66.60		
					Total Area	8.54	651.84		
Weighted So		651.8	=	76.4					
Complex Nu	mber	8.5							
					ASSUMES 5 MINUT	E TIME OF	CONCENTRATION		

Civil Er	Well Land Planning			SOIL CONSERVATION SERVICE HYDROLOGIC DATA FOR WATERSHED RUNOFF COMPUTATIONS					
www.L	DLHowell.co	m						DATE: BY:	<u>3/29/2021</u> <u>DWG</u>
JOB NO.: DESCRIPTI		PROJECT: POST DEVELOPED B	Stokes Estate SYPASS DP002				TOWNSHIP:	<u>Westtown</u>	
Total Area:	1.54 a								
Cumb al	Soil Name	Hydrological	Land Use	Hydrologic	Soil Runoff	Area	Complex Number	Comment	
Symbol GdB	Gladstone	Soil Group	Meadow	Condition Good	Curve Number 58	acres	0.00		
Gab	Loam	B	Lawn	Good	58 61	0.00 1.54	93.94		
	Loam	ы N/A	Impervious	N/A	98	0.00	93.94 0.00		
Ва	Baile Silt	C	Meadow	Good	50 71	0.00	0.00		
Da	Loam	c	Lawn	Good	74	0.00	0.00		
					Total Area	1.54	93.94		
Weighted So	bil	93.9	=	61.0					
Complex Nu		1.5			1				
					ASSUMES 5 MINUT	E TIME OF	CONCENTRATION		

APPENDIX D

HYDRAFLOW HYDROGRAPH REPORTS

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100 - Year

Hydrograph Return Period Recap Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

lo. 1	type (origin)	hyd(s))			Hydrograph
1			1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	Description
'	SCS Runoff		1.295	4.322		12.58	20.75	33.98	46.28	60.31	Pre Developed DP001
2	SCS Runoff		2.474	6.847		18.02	28.64	45.70	61.46	79.34	Pre Developed DP002
4	SCS Runoff		11.58	17.70		29.68	39.83	55.13	68.94	84.16	Post Basin 3
5	Reservoir	4	0.000	0.000		0.543	1.424	6.253	19.64	24.38	Basin 3 Routed
6	SCS Runoff		0.583	1.146		2.314	3.349	4.966	6.425	8.052	Post Bypass DP001
7	Combine	5, 6	0.583	1.146		2.314	3.349	6.859	21.53	29.04	Post Total DP001
9	SCS Runoff		11.30	16.97		28.01	37.29	51.31	63.94	77.75	Post Basin 1
10	Reservoir	9	0.000	0.000		0.510	1.244	3.633	10.69	30.18	Basin 1 Upper Routed
11	Reservoir	10	0.000	0.000		0.375	0.928	2.071	5.226	13.13	Basin 1 Lower Routed
12	SCS Runoff		12.13	17.09		26.44	34.26	46.02	56.20	67.22	Post Basin 2
13	Reservoir	12	0.000	0.120		0.682	1.706	5.509	18.98	33.12	Basin 2 Routed
14	SCS Runoff		0.417	0.956		2.121	3.170	4.826	6.335	8.028	Post Bypass DP002
15	Combine	11, 13, 14	0.417	0.956		2.121	3.170	6.169	21.32	37.69	Post Total DP002

Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	1.295	2	726	10,628				Pre Developed DP001
2	SCS Runoff	2.474	2	724	15,638				Pre Developed DP002
4	SCS Runoff	11.58	2	718	24,233				Post Basin 3
5	Reservoir	0.000	2	n/a	0	4	317.29	24,233	Basin 3 Routed
6	SCS Runoff	0.583	2	718	1,570				Post Bypass DP001
7	Combine	0.583	2	718	1,570	5, 6			Post Total DP001
9	SCS Runoff	11.30	2	718	23,379				Post Basin 1
10	Reservoir	0.000	2	n/a	0	9	321.56	23,379	Basin 1 Upper Routed
11	Reservoir	0.000	2	n/a	0	10	302.20	0.000	Basin 1 Lower Routed
12	SCS Runoff	12.13	2	718	24,366				Post Basin 2
13	Reservoir	0.000	2	n/a	0	12	308.25	24,366	Basin 2 Routed
14	SCS Runoff	0.417	2	720	1,355				Post Bypass DP002
15	Combine	0.417	2	720	1,355	11, 13, 14			Post Total DP002
SW	M.gpw				Return I	Period: 1 Ye	ear	Thursday,	09 / 30 / 2021

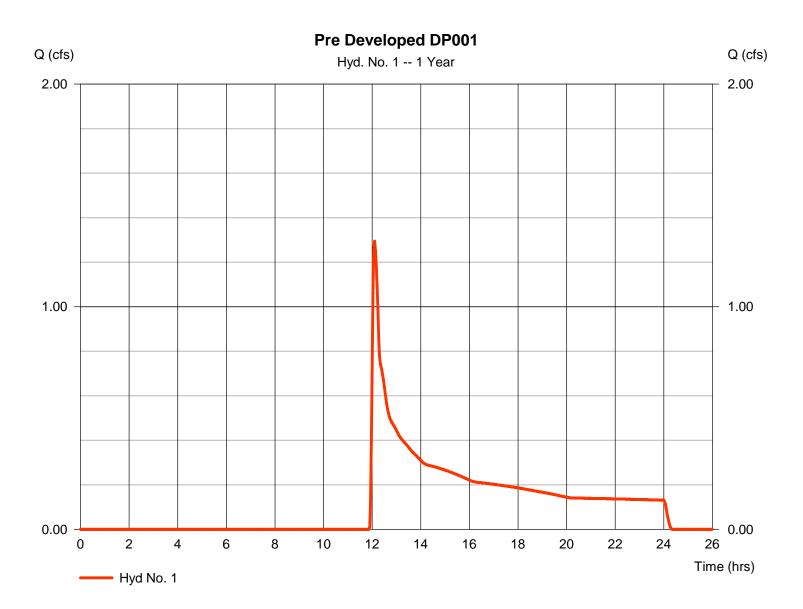
Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 1

Pre Developed DP001

Hydrograph type	= SCS Runoff	Peak discharge	= 1.295 cfs
Storm frequency	= 1 yrs	Time to peak	= 12.10 hrs
Time interval	= 2 min	Hyd. volume	= 10,628 cuft
Drainage area	= 15.430 ac	Curve number	= 58
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= TR55	Time of conc. (Tc)	= 12.00 min
Total precip.	= 2.70 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



3

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 1

Pre Developed DP001

Description	A		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.240 = 100.0 = 3.20 = 4.00		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 10.81	+	0.00	+	0.00	=	10.81
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 330.00 = 9.00 = Unpaved =4.84	ł	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 1.14	+	0.00	+	0.00	=	1.14
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015 0.00		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							12.00 min

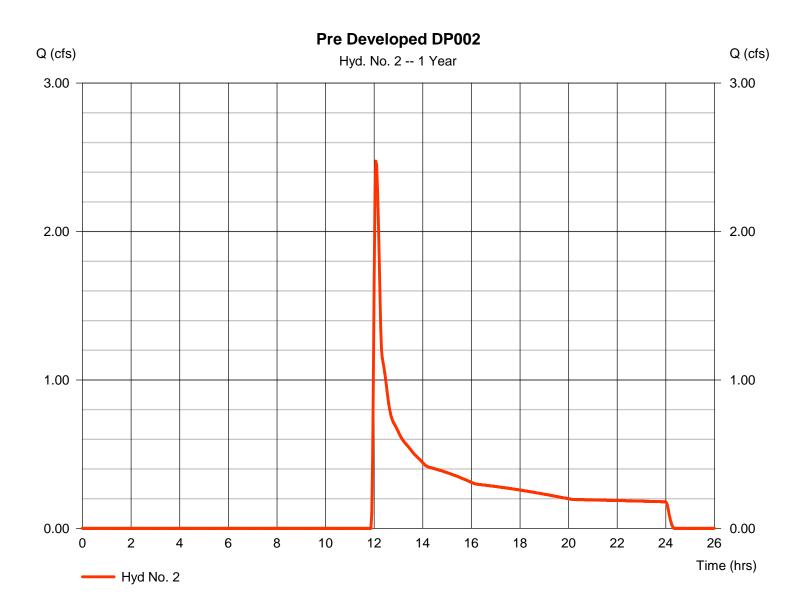
Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 2

Pre Developed DP002

Hydrograph type	= SCS Runoff	Peak discharge	= 2.474 cfs
Storm frequency	= 1 yrs	Time to peak	= 12.07 hrs
Time interval	= 2 min	Hyd. volume	= 15,638 cuft
Drainage area	= 19.210 ac	Curve number	= 59.4
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= TR55	Time of conc. (Tc)	= 12.00 min
Total precip.	= 2.70 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 2

Pre Developed DP002

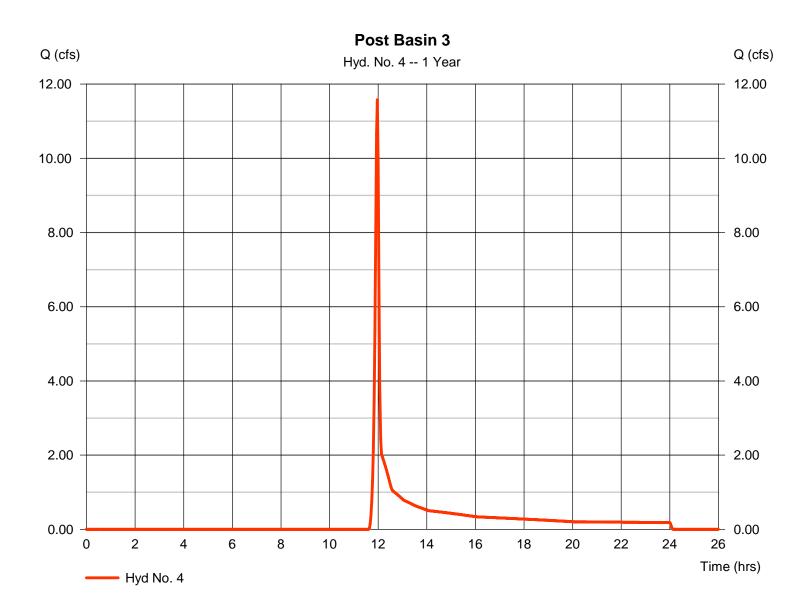
Description	A		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.240 = 100.0 = 3.20 = 7.00		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 8.65	+	0.00	+	0.00	=	8.65
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 957.00 = 8.90 = Unpaved =4.81	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 3.31	+	0.00	+	0.00	=	3.31
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015 0.00		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							12.00 min

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 4

Post Basin 3

Hydrograph type	= SCS Runoff	Peak discharge	= 11.58 cfs
Storm frequency	= 1 yrs	Time to peak	= 11.97 hrs
Time interval	= 2 min	Hyd. volume	= 24,233 cuft
Drainage area	= 12.150 ac	Curve number	= 70.8
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 2.70 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



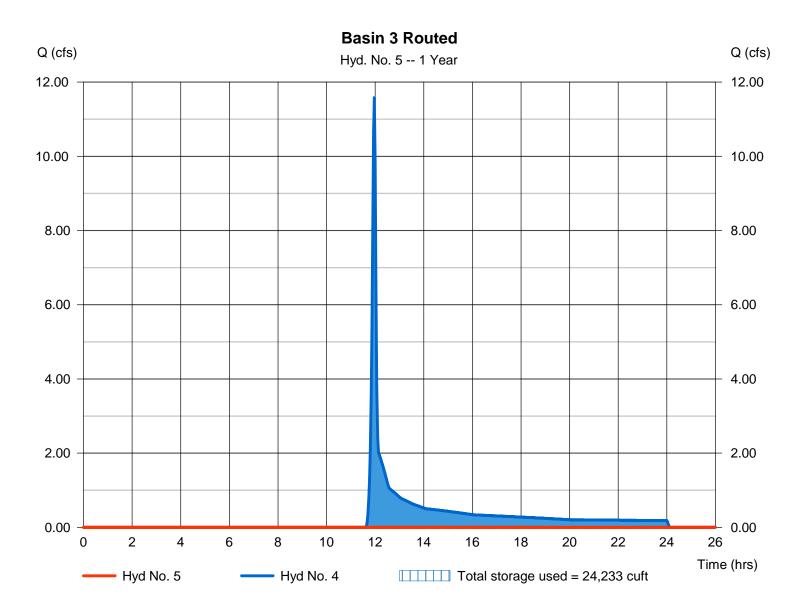
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 5

Basin 3 Routed

Hydrograph type	= Reservoir	Peak discharge	= 0.000 cfs
Storm frequency	= 1 yrs	Time to peak	= n/a
Time interval	= 2 min	Hyd. volume	= 0 cuft
Inflow hyd. No.	= 4 - Post Basin 3	Max. Elevation	= 317.29 ft
Reservoir name	= Basin 3	Max. Storage	= 24,233 cuft

Storage Indication method used.



Pond Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Pond Data

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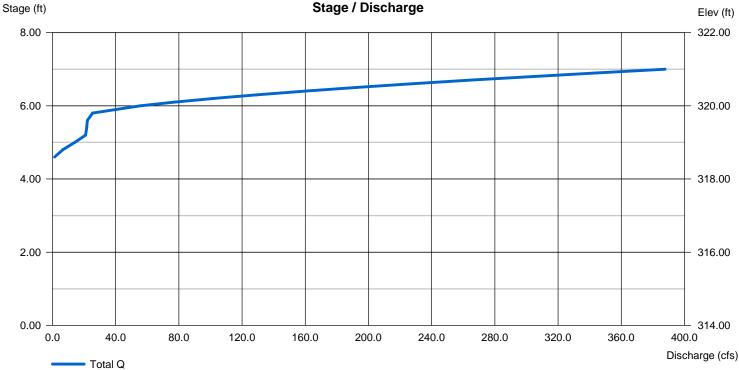
Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	314.00	2,000	0	0
0.20	314.20	2,000	159	159
0.40	314.40	2,000	159	318
0.60	314.60	2,000	159	478
0.80	314.80	2,000	159	637
1.00	314.99	2,000	159	796
1.19	315.19	2,000	159	955
1.39	315.39	2,000	159	1,114
1.59	315.59	2,000	159	1,274
1.79	315.79	2,000	159	1,433
1.99	315.99	2,000	159	1,592
2.00	316.00	15,345	87	1,679
4.00	318.00	19,665	35,010	36,689
6.00	320.00	24,211	43,876	80,565
7.00	321.00	26,570	25,391	105,955

Culvert / Orifice Structures

[A] [B] [C] [PrfRsr] [A] [B] [C] [D] Rise (in) = 18.00 Inactive 0.00 0.00 Crest Len (ft) = 12.00 Inactive 100.00 0.00 Span (in) = 18.00 5.00 0.00 0.00 Crest El. (ft) = 318.50 318.00 319.75 0.00 No. Barrels = 1 0 Weir Coeff. = 3.33 3.33 2.60 3.33 0 1 Broad Invert El. (ft) = 312.00 318.00 0.00 0.00 Weir Type Rect = 1 ----Length (ft) = 50.00 0.00 0.00 0.00 Multi-Stage = Yes Yes No No Slope (%) = 5.00 0.00 0.00 n/a = .013 .013 .013 N-Value n/a Orifice Coeff. = 0.60 0.60 0.60 0.60 Exfil.(in/hr) = 0.000 (by Wet area) = n/a Yes No No TW Elev. (ft) = 0.00 Multi-Stage

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).



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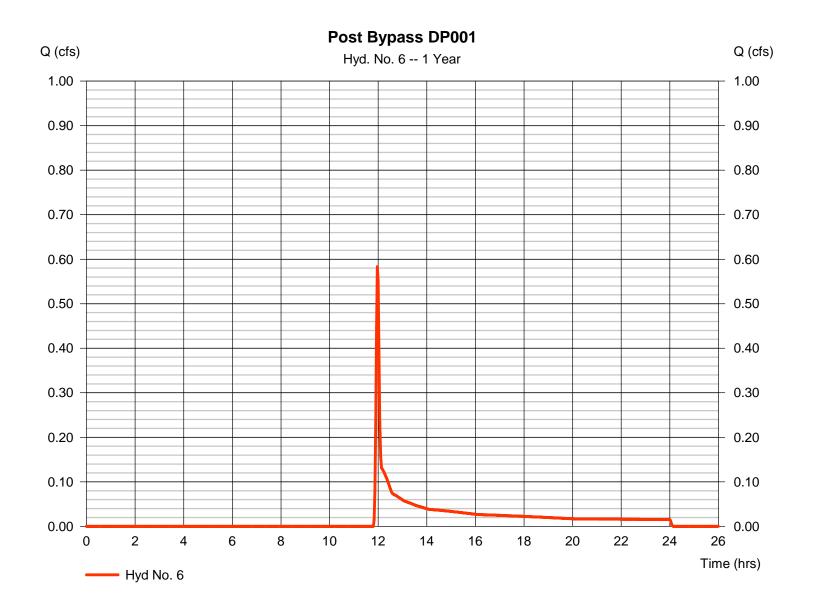
Weir Structures

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 6

Post Bypass DP001

Hydrograph type	= SCS Runoff	Peak discharge	= 0.583 cfs
Storm frequency	= 1 yrs	Time to peak	= 11.97 hrs
Time interval	= 2 min	Hyd. volume	= 1,570 cuft
Drainage area	= 1.440 ac	Curve number	= 63.2
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 2.70 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

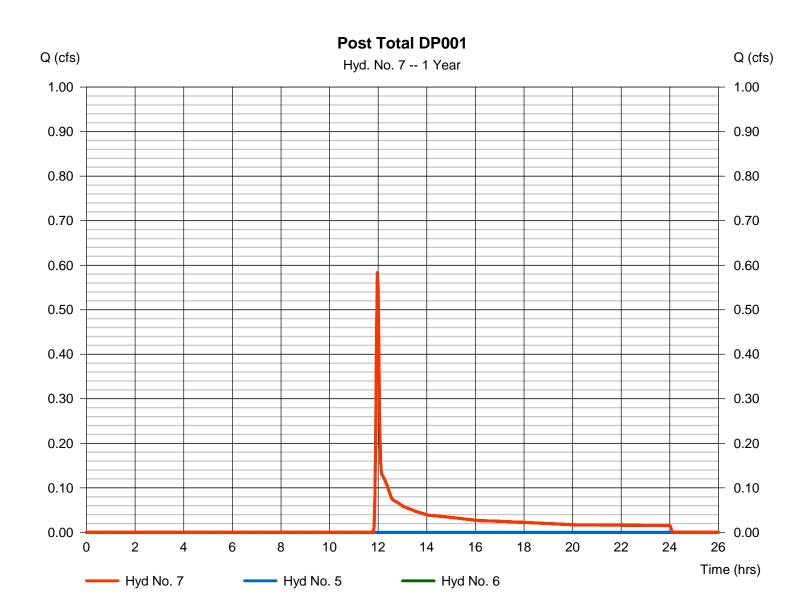


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 7

Post Total DP001

Hydrograph type Storm frequency Time interval	= Combine = 1 yrs = 2 min	Peak discharge Time to peak Hyd. volume	= 0.583 cfs = 11.97 hrs = 1,570 cuft = 1,440 pc
Inflow hyds.	= 5, 6	Contrib. drain. area	= 1.440 ac

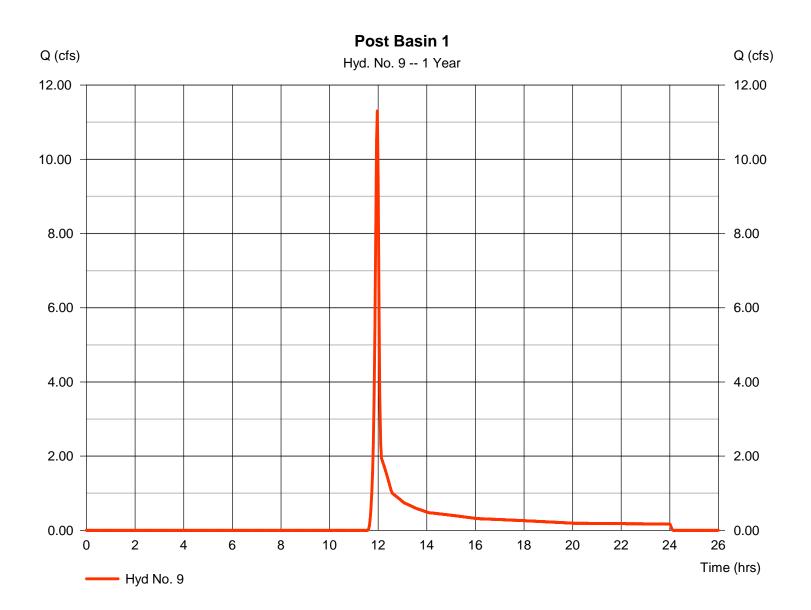


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 9

Post Basin 1

Hydrograph type	= SCS Runoff	Peak discharge	= 11.30 cfs
Storm frequency	= 1 yrs	Time to peak	= 11.97 hrs
Time interval	= 2 min	Hyd. volume	= 23,379 cuft
Drainage area	= 10.950 ac	Curve number	= 71.8
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 2.70 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



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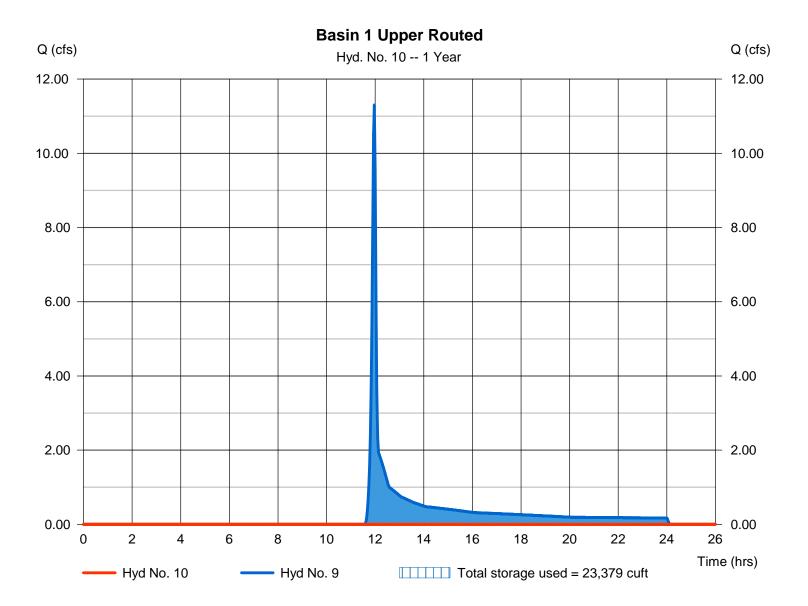
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 10

Basin 1 Upper Routed

Hydrograph type	= Reservoir	Peak discharge	= 0.000 cfs
Storm frequency	= 1 yrs	Time to peak	= n/a
Time interval	= 2 min	Hyd. volume	= 0 cuft
Inflow hyd. No.	= 9 - Post Basin 1	Max. Elevation	= 321.56 ft
Reservoir name	= Basin 1 Upper	Max. Storage	= 23,379 cuft
Reservoir name		Max. Storage	= 25,579 cuit

Storage Indication method used.



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Pond Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Pond No. 2 - Basin 1 Upper

Pond Data

Contours -User-defined contour areas. Average end area method used for volume calculation. Begining Elevation = 320.00 ft

Stage / Storage Table

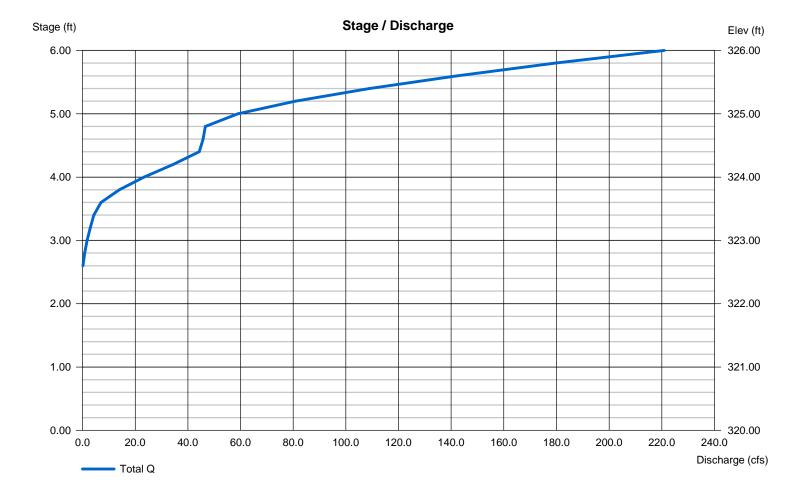
Stage (ft)	Elevation (1	ft)	Contour a	irea (sqft)	Incr. Storage (cuft)	Total sto	rage (cuft)		
0.00	320.00		13,236	6	0		0		
2.00	322.00		16,755	5	29,991	29,9	991		
4.00	324.00		20,617	,	37,372	67,	363		
6.00	326.00		24,500)	45,117	112,	480		
Culvert / O	rifice Structure	es			Weir Structu	ires			
	[A]	[B]	[C]	[PrfRsr]		[A]	[B]	[C]	[D]
Rise (in)	= 24.00	0.00	0.00	0.00	Crest Len (ft)	= 12.00	1.50	50.00	0.00
Span (in)	= 24.00	0.00	0.00	0.00	Crest El. (ft)	= 323.50	322.50	324.80	0.00

No. Barrels	= 1	0	0	0	Weir Coeff.	= 3.33	3.33	2.60
Invert El. (ft)	= 314.00	0.00	0.00	0.00	Weir Type	= 1	Rect	Broad
Length (ft)	= 50.00	0.00	0.00	0.00	Multi-Stage	= Yes	Yes	No
Slope (%)	= 0.50	0.00	0.00	n/a				
N-Value	= .013	.013	.013	n/a				
Orifice Coeff.	= 0.60	0.60	0.60	0.60	Exfil.(in/hr)	= 0.000 (by	/ Contour)	
Multi-Stage	= n/a	No	No	No	TW Elev. (ft)	= 0.00		

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).

3.33

----No



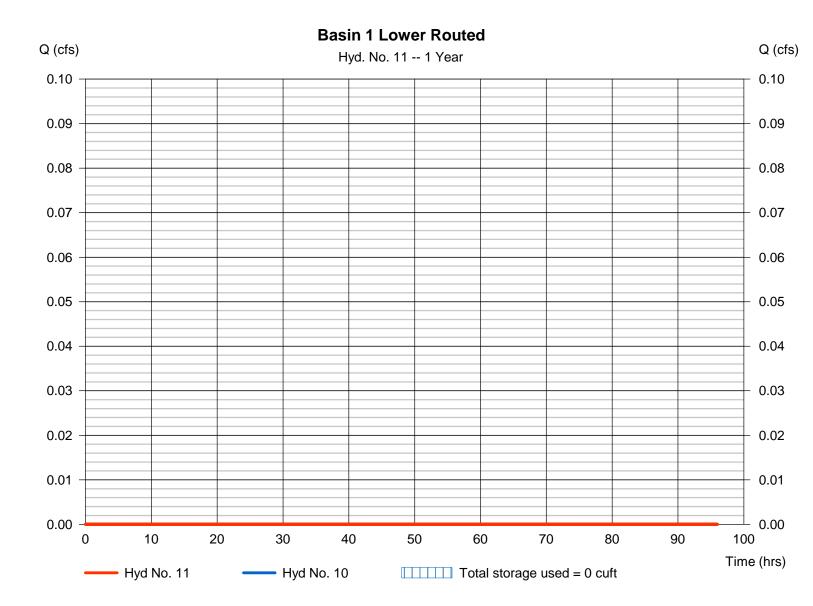
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 11

Basin 1 Lower Routed

Hydrograph type	= Reservoir	Peak discharge	= 0.000 cfs
Storm frequency	= 1 yrs	Time to peak	= n/a
Time interval	= 2 min	Hyd. volume	= 0 cuft
Inflow hyd. No.	= 10 - Basin 1 Upper Routed	Max. Elevation	= 302.20 ft
Reservoir name	= Basin 1 Lower	Max. Storage	= 0 cuft

Storage Indication method used.



Thursday, 09 / 30 / 2021

Pond Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Pond No. 1 - Basin 1 Lower

Pond Data

Contours -User-defined contour areas. Average end area method used for volume calculation. Begining Elevation = 302.00 ft

Stage / Storage Table

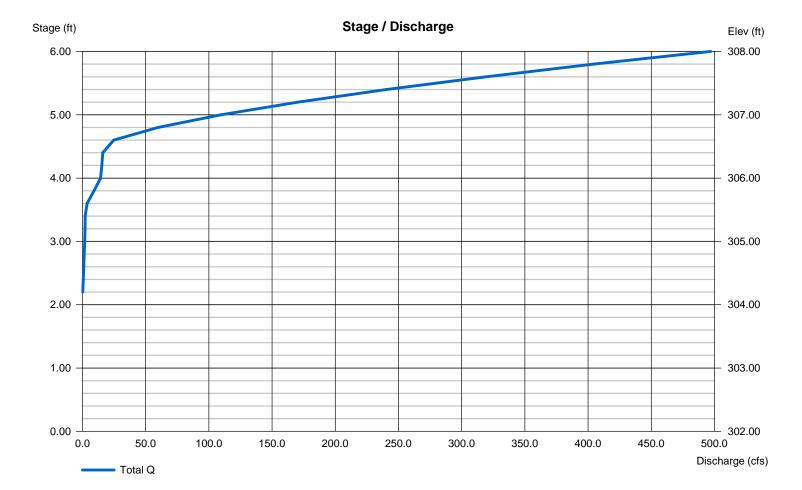
Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	302.00	2,215	0	0
2.00	304.00	3,972	6,187	6,187
4.00	306.00	10,535	14,507	20,694
6.00	308.00	14,870	25,405	46,099

Weir Structures

Culvert / Orifice Structures

	[A]	[B]	[C]	[PrfRsr]		[A]	[B]	[C]	[D]
Rise (in)	= 18.00	9.00	0.00	0.00	Crest Len (ft)	= 12.00	100.00	0.00	0.00
Span (in)	= 18.00	9.00	0.00	0.00	Crest El. (ft)	= 305.50	306.50	0.00	0.00
No. Barrels	= 1	1	0	0	Weir Coeff.	= 3.33	2.60	3.33	3.33
Invert El. (ft)	= 302.00	304.00	0.00	0.00	Weir Type	= 1	Broad		
Length (ft)	= 50.00	0.00	0.00	0.00	Multi-Stage	= Yes	No	No	No
Slope (%)	= 0.50	0.00	0.00	n/a					
N-Value	= .013	.013	.013	n/a					
Orifice Coeff.	= 0.60	0.60	0.60	0.60	Exfil.(in/hr)	= 0.000 (by	Contour)		
Multi-Stage	= n/a	Yes	No	No	TW Elev. (ft)	= 0.00			

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).

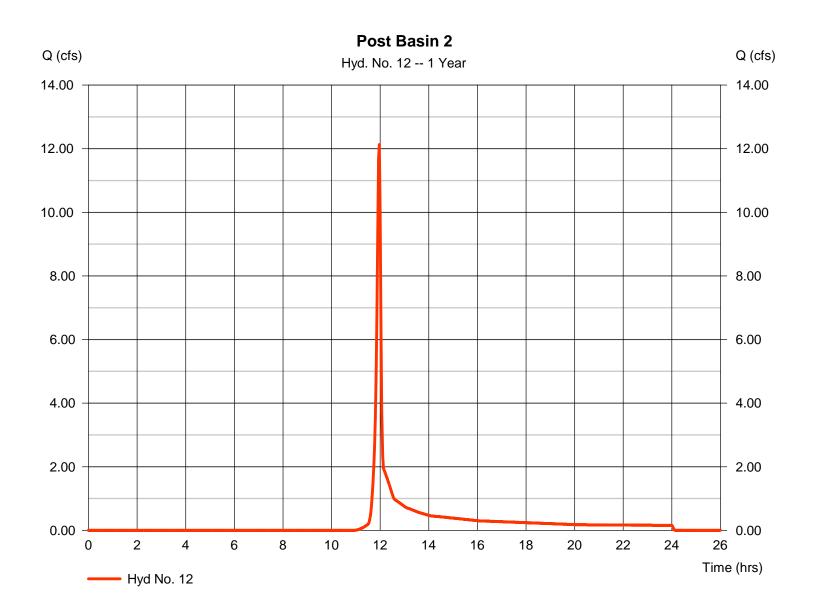


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 12

Post Basin 2

Hydrograph type	= SCS Runoff	Peak discharge	= 12.13 cfs
Storm frequency	= 1 yrs	Time to peak	= 11.97 hrs
Time interval	= 2 min	Hyd. volume	= 24,366 cuft
Drainage area	= 8.540 ac	Curve number	= 76.4
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 2.70 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



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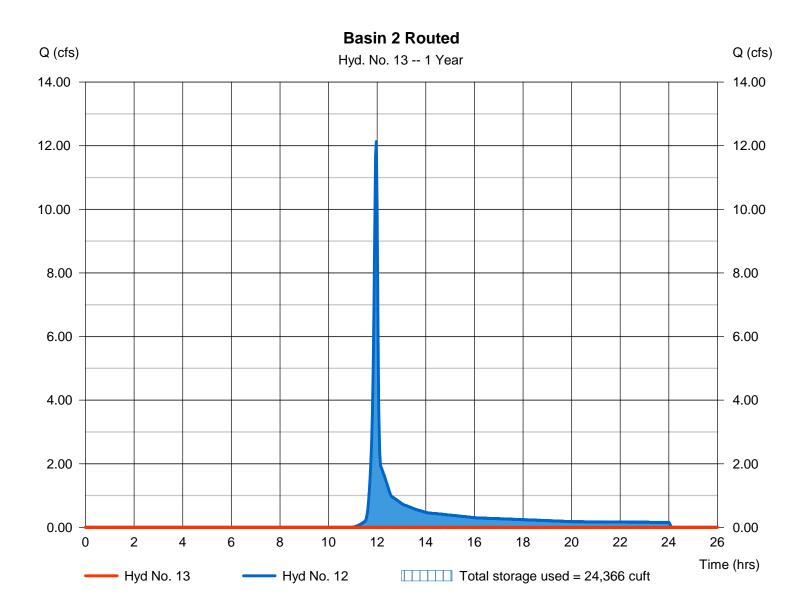
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 13

Basin 2 Routed

Hydrograph type	= Reservoir	Peak discharge	= 0.000 cfs
Storm frequency	= 1 yrs	Time to peak	= n/a
Time interval	= 2 min	Hyd. volume	= 0 cuft
Inflow hyd. No.	= 12 - Post Basin 2	Max. Elevation	= 308.25 ft
Reservoir name	= Basin 2	Max. Storage	= 24,366 cuft

Storage Indication method used.



Pond Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

.013

n/a

.013

Pond No. 3 - Basin 2

Pond Data

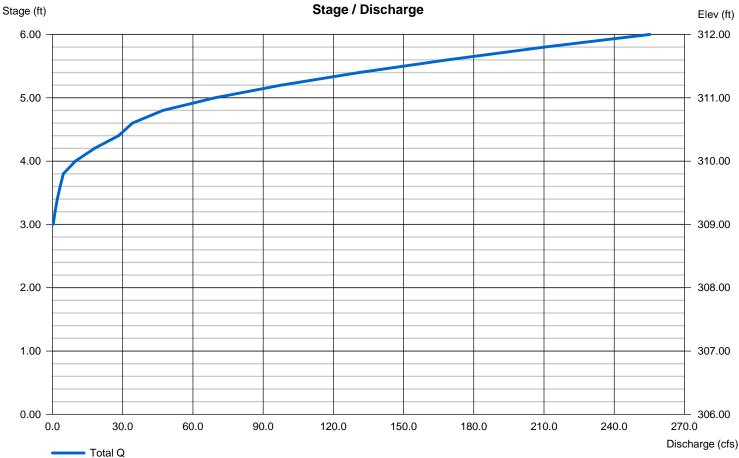
Contours -User-defined contour areas. Average end area method used for volume calculation. Begining Elevation = 306.00 ft

Stage / Storage Table

olage / olon	age rabie								
Stage (ft)	Elevation (f	t)	Contour a	rea (sqft)	Incr. Storage (cuft)	Total sto	rage (cuft)		
0.00	306.00		8,255	5	0		0		
2.00	308.00		12,443	3	20,698	20,0	698		
4.00	310.00		16,673	3	29,116	49,	814		
6.00	312.00		21,110		37,783	87,	597		
Culvert / Ori	fice Structure	S			Weir Structu	ires			
	[A]	[B]	[C]	[PrfRsr]		[A]	[B]	[C]	[D]
Rise (in)	= 24.00	0.00	0.00	0.00	Crest Len (ft)	= 12.00	1.50	50.00	0.00
Span (in)	= 24.00	0.00	0.00	0.00	Crest El. (ft)	= 309.80	308.85	310.60	0.00
No. Barrels	= 1	0	0	0	Weir Coeff.	= 3.33	3.33	2.60	3.33
Invert El. (ft)	= 304.00	0.00	0.00	0.00	Weir Type	= 1	Rect	Broad	
Length (ft)	= 50.00	0.00	0.00	0.00	Multi-Stage	= Yes	Yes	No	No
Slope (%)	= 0.50	0.00	0.00	n/a					

= 0.60 0.60 0.60 = 0.000 (by Contour) 0.60 Exfil.(in/hr) = n/a No No No TW Elev. (ft) = 0.00

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).



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N-Value

Orifice Coeff.

Multi-Stage

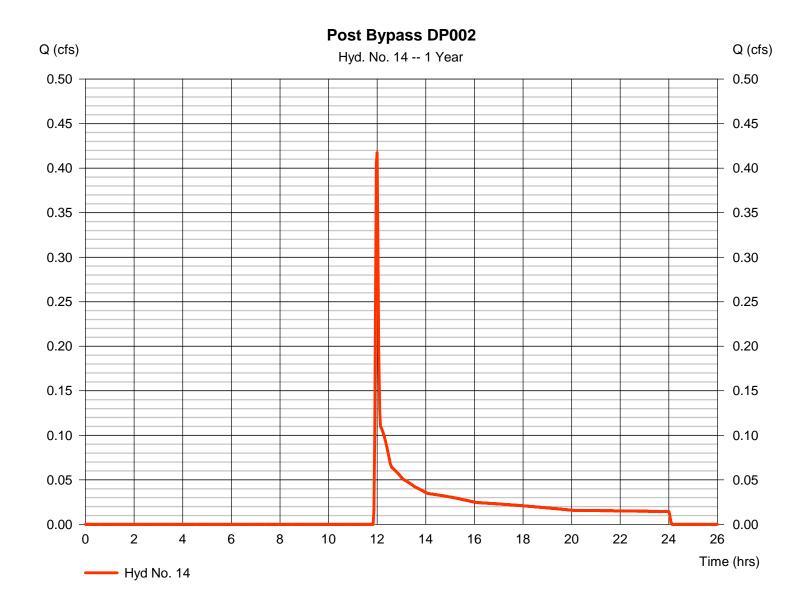
= .013

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 14

Post Bypass DP002

Hydrograph type	= SCS Runoff	Peak discharge	= 0.417 cfs
Storm frequency	= 1 yrs	Time to peak	= 12.00 hrs
Time interval	= 2 min	Hyd. volume	= 1,355 cuft
Drainage area	= 1.540 ac	Curve number	= 61
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 2.70 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484
Storm duration	= 24 nrs	Shape factor	= 484



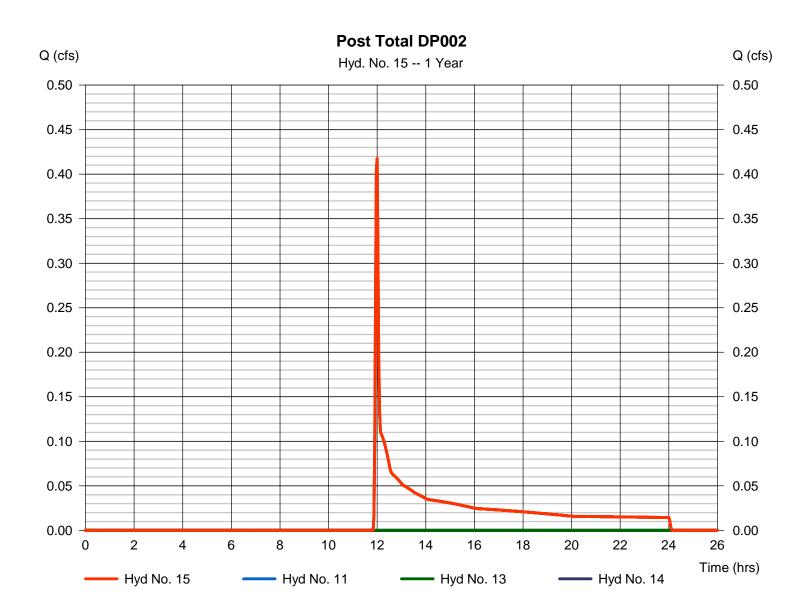
20

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 15

Post Total DP002

Hydrograph type	Combine1 yrs	Peak discharge	= 0.417 cfs
Storm frequency		Time to peak	= 12.00 hrs
Time interval	= 2 min	Hyd. volume	= 1,355 cuft
Inflow hyds.	= 11, 13, 14	Contrib. drain. area	= 1.540 ac



Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

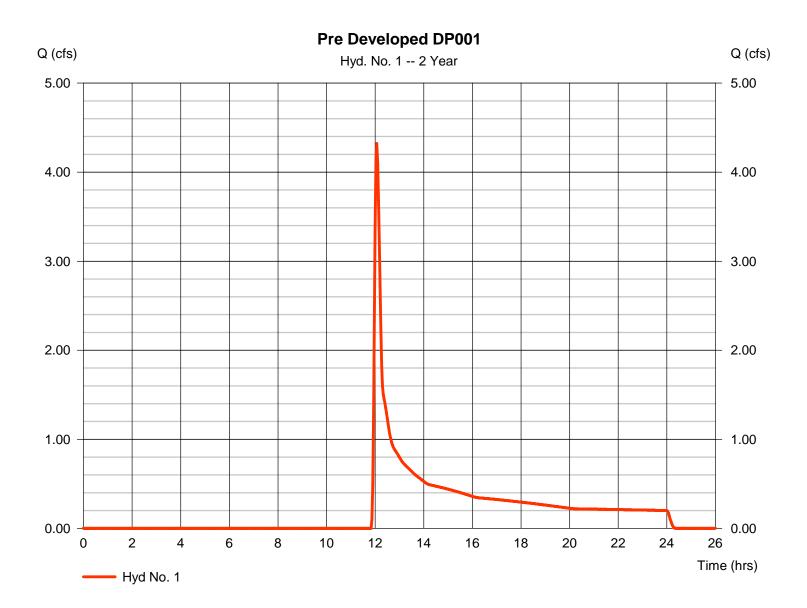
Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	4.322	2	724	19,687				Pre Developed DP001
2	SCS Runoff	6.847	2	724	27,873				Pre Developed DP002
4	SCS Runoff	17.70	2	718	35,889				Post Basin 3
5	Reservoir	0.000	2	n/a	0	4	317.95	35,889	Basin 3 Routed
6	SCS Runoff	1.146	2	718	2,583				Post Bypass DP001
7	Combine	1.146	2	718	2,583	5, 6			Post Total DP001
9	SCS Runoff	16.97	2	718	34,254				Post Basin 1
10	Reservoir	0.000	2	n/a	0	9	322.23	34,254	Basin 1 Upper Routed
11	Reservoir	0.000	2	n/a	0	10	302.20	0.000	Basin 1 Lower Routed
12	SCS Runoff	17.09	2	718	34,170				Post Basin 2
13	Reservoir	0.120	2	1444	1,815	12	308.88	33,546	Basin 2 Routed
14	SCS Runoff	0.956	2	718	2,327				Post Bypass DP002
15	Combine	0.956	2	718	4,142	11, 13, 14			Post Total DP002
SW	/M.gpw				Return I	Period: 2 Ye	ar	Thursday,	09 / 30 / 2021

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 1

Pre Developed DP001

Hydrograph type	= SCS Runoff	Peak discharge	= 4.322 cfs
Storm frequency	= 2 yrs	Time to peak	= 12.07 hrs
Time interval	= 2 min	Hyd. volume	= 19,687 cuft
Drainage area	= 15.430 ac	Curve number	= 58
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= TR55	Time of conc. (Tc)	= 12.00 min
Total precip.	= 3.20 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

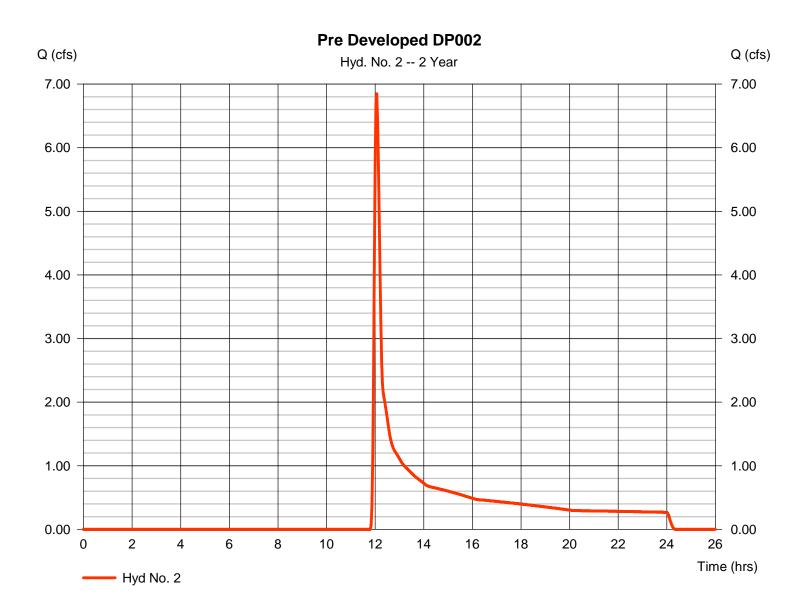


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 2

Pre Developed DP002

Hydrograph type	= SCS Runoff	Peak discharge	= 6.847 cfs
Storm frequency	= 2 yrs	Time to peak	= 12.07 hrs
Time interval	= 2 min	Hyd. volume	= 27,873 cuft
Drainage area	= 19.210 ac	Curve number	= 59.4
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= TR55	Time of conc. (Tc)	= 12.00 min
Total precip.	= 3.20 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

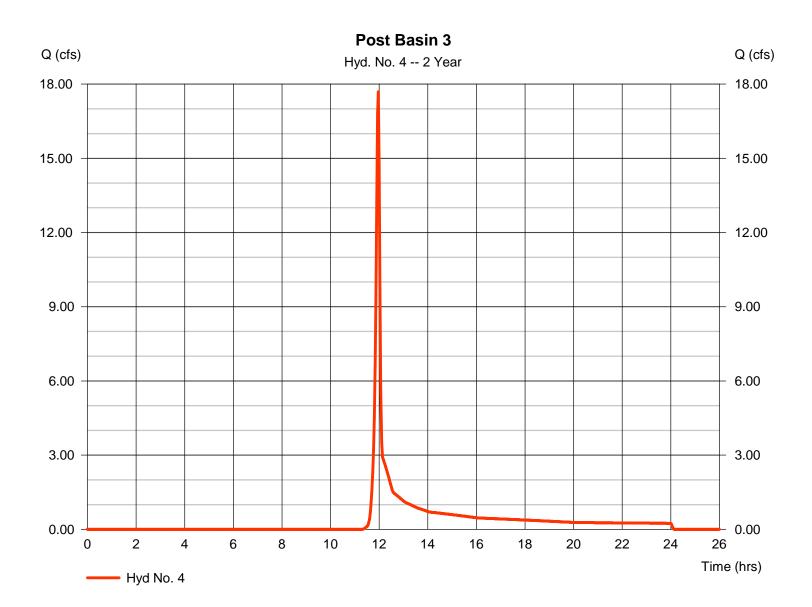


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 4

Post Basin 3

Hydrograph type	= SCS Runoff	Peak discharge	= 17.70 cfs
Storm frequency	= 2 yrs	Time to peak	= 11.97 hrs
Time interval	= 2 min	Hyd. volume	= 35,889 cuft
Drainage area	= 12.150 ac	Curve number	= 70.8
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 3.20 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



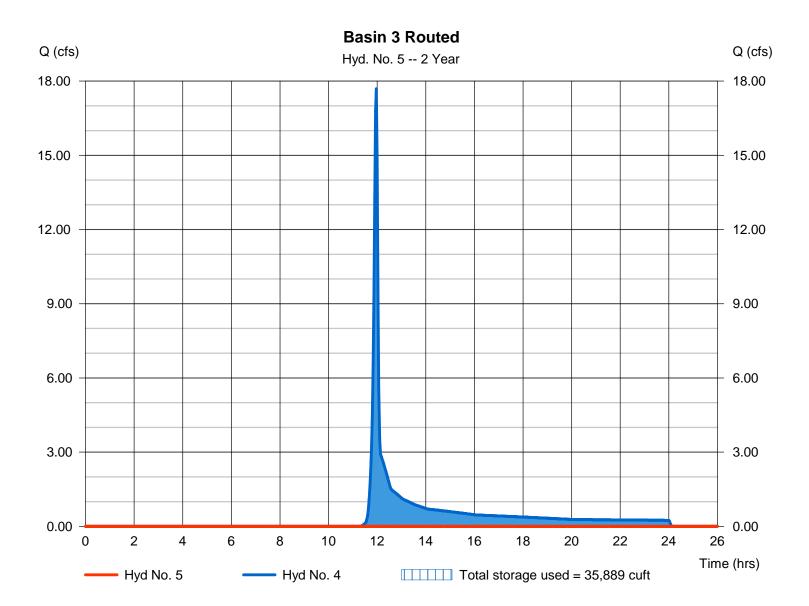
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 5

Basin 3 Routed

Hydrograph type	= Reservoir	Peak discharge	= 0.000 cfs
Storm frequency	= 2 yrs	Time to peak	= n/a
Time interval	= 2 min	Hyd. volume	= 0 cuft
Inflow hyd. No.	= 4 - Post Basin 3	Max. Elevation	= 317.95 ft
Reservoir name	= Basin 3	Max. Storage	= 35,889 cuft

Storage Indication method used.



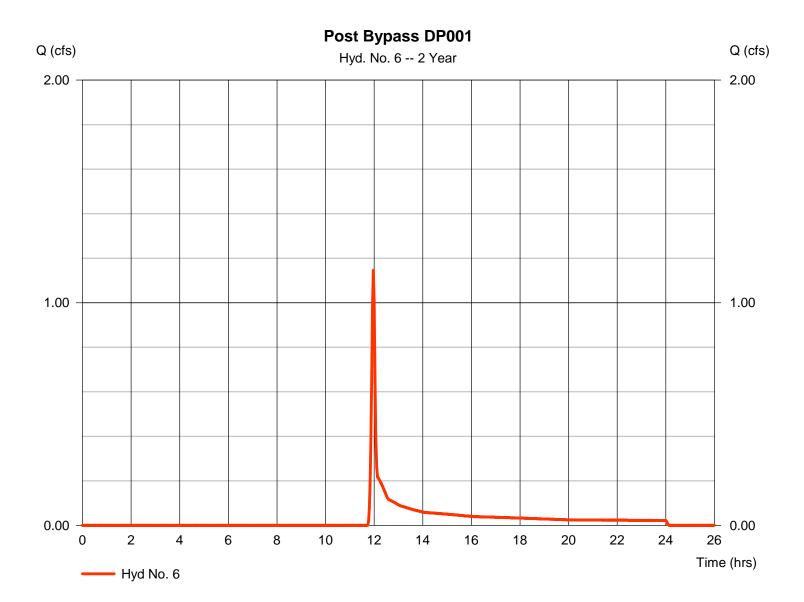
26

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 6

Post Bypass DP001

Hydrograph type	= SCS Runoff	Peak discharge	= 1.146 cfs
Storm frequency	= 2 yrs	Time to peak	= 11.97 hrs
Time interval	= 2 min	Hyd. volume	= 2,583 cuft
Drainage area	= 1.440 ac	Curve number	= 63.2
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 3.20 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

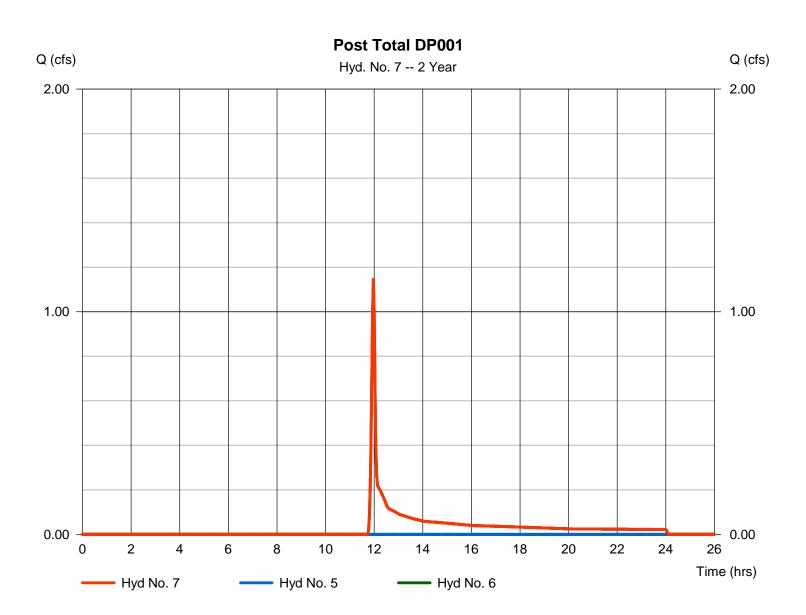


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 7

Post Total DP001

Hydrograph type Storm frequency Time interval Inflow hyds.	 Combine 2 yrs 2 min 5, 6 	Peak discharge Time to peak Hyd. volume Contrib. drain. area	 = 1.146 cfs = 11.97 hrs = 2,583 cuft = 1.440 ac
Inflow hyds.	= 5,6	Contrib. drain. area	= 1.440 ac

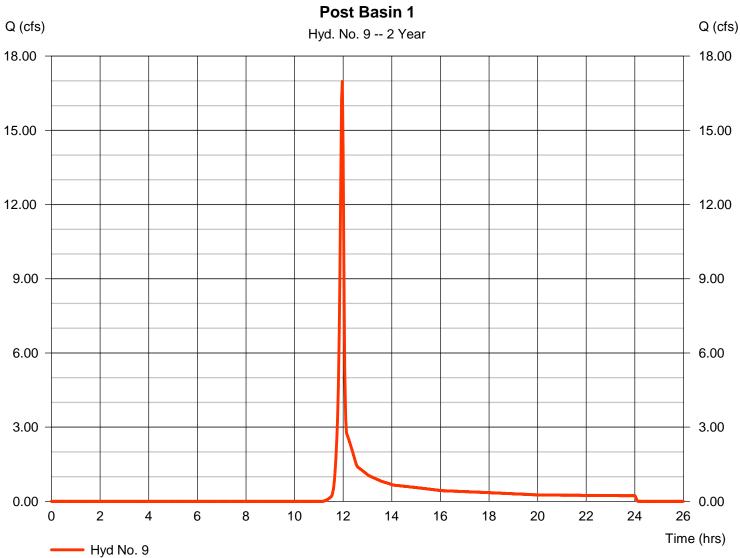


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 9

Post Basin 1

Hydrograph type	= SCS Runoff	Peak discharge	= 16.97 cfs
Storm frequency	= 2 yrs	Time to peak	= 11.97 hrs
Time interval	= 2 min	Hyd. volume	= 34,254 cuft
Drainage area	= 10.950 ac	Curve number	= 71.8
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 3.20 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



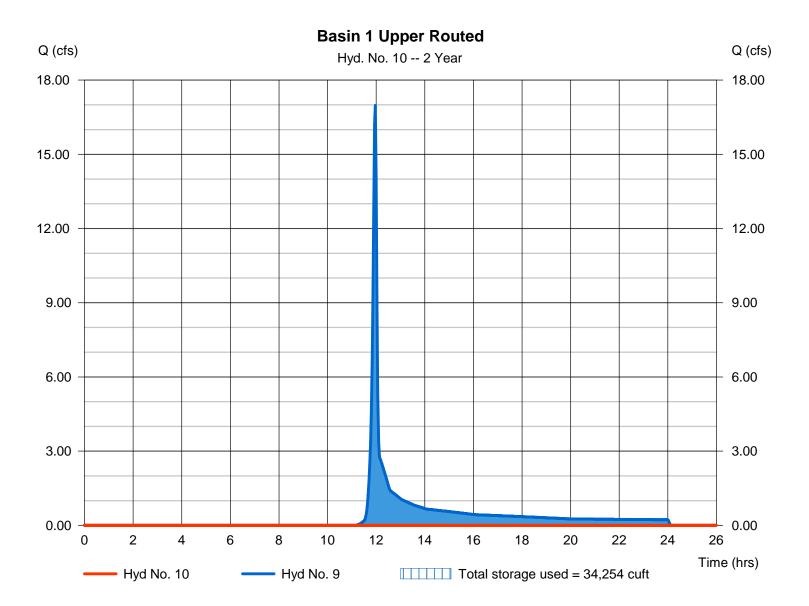
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 10

Basin 1 Upper Routed

Hydrograph type	= Reservoir	Peak discharge	= 0.000 cfs
Storm frequency	= 2 yrs	Time to peak	= n/a
Time interval	= 2 min	Hyd. volume	= 0 cuft
Inflow hyd. No.	= 9 - Post Basin 1	Max. Elevation	= 322.23 ft
Reservoir name	= Basin 1 Upper	Max. Storage	= 34,254 cuft

Storage Indication method used.



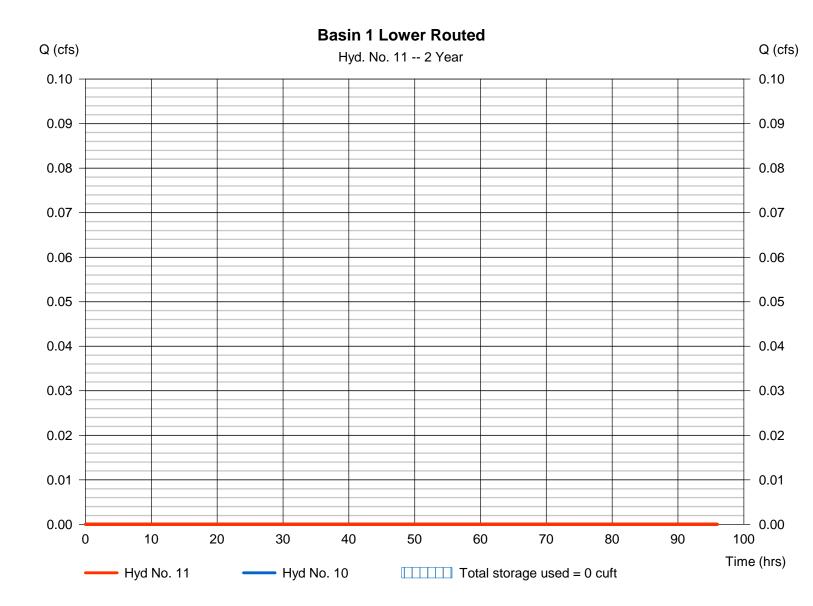
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 11

Basin 1 Lower Routed

Hydrograph type	= Reservoir	Peak discharge	= 0.000 cfs
Storm frequency	= 2 yrs	Time to peak	= n/a
Time interval	= 2 min	Hyd. volume	= 0 cuft
Inflow hyd. No.	= 10 - Basin 1 Upper Routed	Max. Elevation	= 302.20 ft
Reservoir name	= Basin 1 Lower	Max. Storage	= 0 cuft

Storage Indication method used.



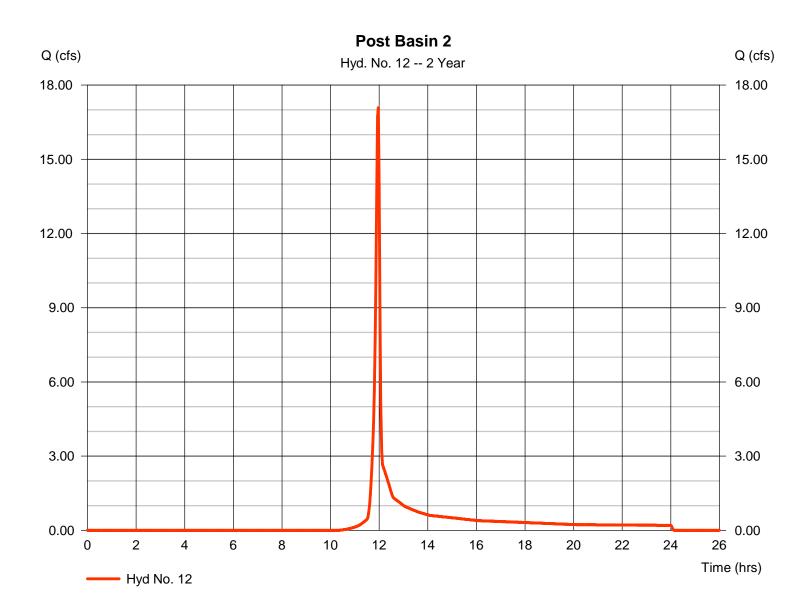
31

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 12

Post Basin 2

Hydrograph type	= SCS Runoff	Peak discharge	= 17.09 cfs
Storm frequency	= 2 yrs	Time to peak	= 11.97 hrs
Time interval	= 2 min	Hyd. volume	= 34,170 cuft
Drainage area	= 8.540 ac	Curve number	= 76.4
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 3.20 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



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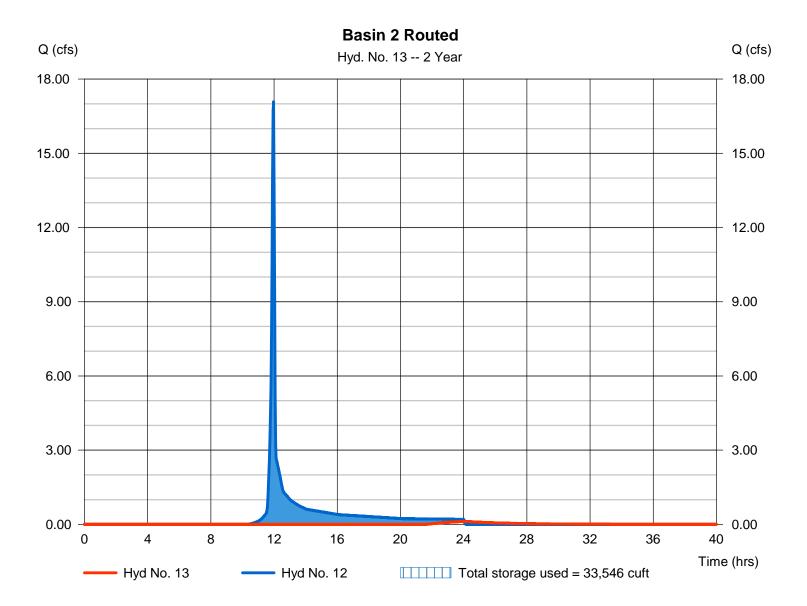
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 13

Basin 2 Routed

Hydrograph type	= Reservoir	Peak discharge	= 0.120 cfs
Storm frequency	= 2 yrs	Time to peak	= 24.07 hrs
Time interval	= 2 min	Hyd. volume	= 1,815 cuft
Inflow hyd. No.	= 12 - Post Basin 2	Max. Elevation	= 308.88 ft
Reservoir name	= Basin 2	Max. Storage	= 33,546 cuft

Storage Indication method used.

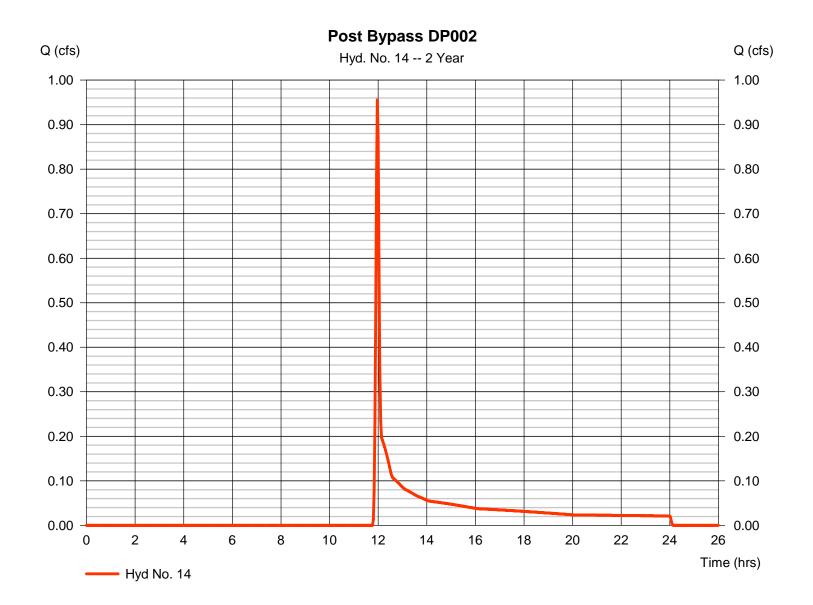


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 14

Post Bypass DP002

Hydrograph type	= SCS Runoff	Peak discharge	= 0.956 cfs
Storm frequency	= 2 yrs	Time to peak	= 11.97 hrs
Time interval	= 2 min	Hyd. volume	= 2,327 cuft
Drainage area	= 1.540 ac	Curve number	= 61
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 3.20 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

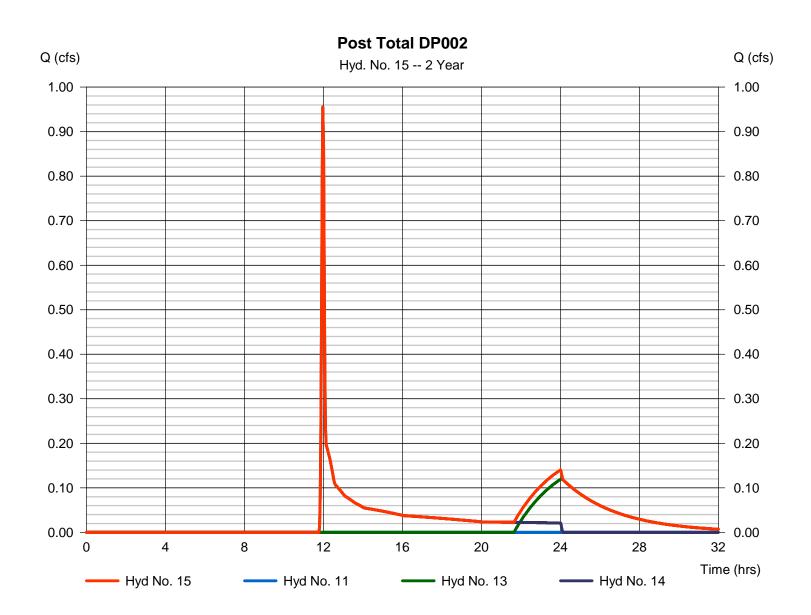


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 15

Post Total DP002

Hydrograph type Storm frequency	Combine2 yrs	Peak discharge Time to peak	= 0.956 cfs = 11.97 hrs
Time interval	= 2 min	Hyd. volume	= 4,142 cuft
Inflow hyds.	= 11, 13, 14	Contrib. drain. area	= 1.540 ac



Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

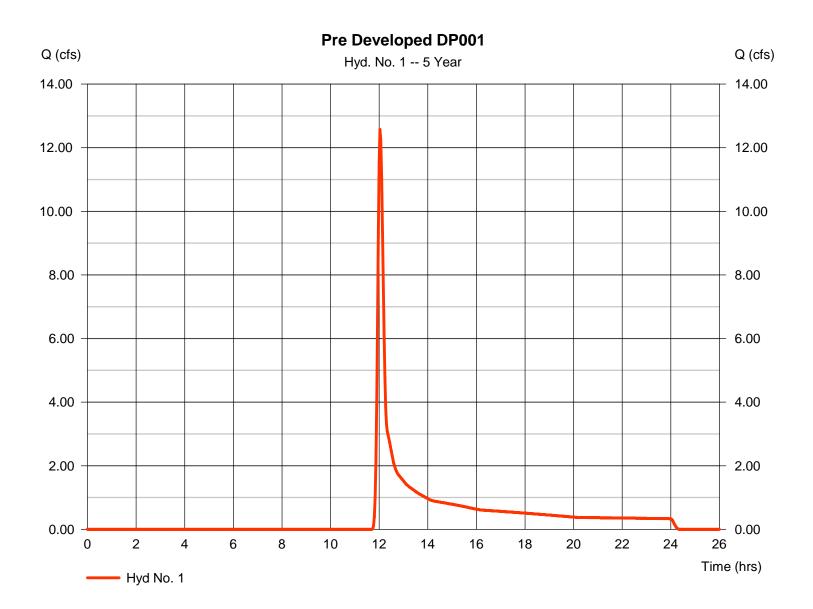
Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	12.58	2	722	40,519				Pre Developed DP001
2	SCS Runoff	18.02	2	722	55,435				Pre Developed DP002
4	SCS Runoff	29.68	2	718	59,371				Post Basin 3
5	Reservoir	0.543	2	1094	13,903	4	318.49	47,347	Basin 3 Routed
6	SCS Runoff	2.314	2	718	4,767				Post Bypass DP001
7	Combine	2.314	2	718	18,670	5, 6			Post Total DP001
9	SCS Runoff	28.01	2	718	56,002				Post Basin 1
10	Reservoir	0.510	2	1086	18,513	9	322.71	43,189	Basin 1 Upper Routed
11	Reservoir	0.375	2	1362	12,317	10	304.32	8,514	Basin 1 Lower Routed
12	SCS Runoff	26.44	2	718	53,176				Post Basin 2
13	Reservoir	0.682	2	918	20,822	12	309.11	36,790	Basin 2 Routed
14	SCS Runoff	2.121	2	718	4,472				Post Bypass DP002
15	Combine	2.121	2	718	37,611	11, 13, 14			Post Total DP002
SW	M.gpw				Return	Period: 5 Ye	ear	Thursday,	09 / 30 / 2021

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 1

Pre Developed DP001

Hydrograph type	= SCS Runoff	Peak discharge	= 12.58 cfs
Storm frequency	= 5 yrs	Time to peak	= 12.03 hrs
Time interval	= 2 min	Hyd. volume	= 40,519 cuft
Drainage area	= 15.430 ac	Curve number	= 58
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= TR55	Time of conc. (Tc)	= 12.00 min
Total precip.	= 4.08 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



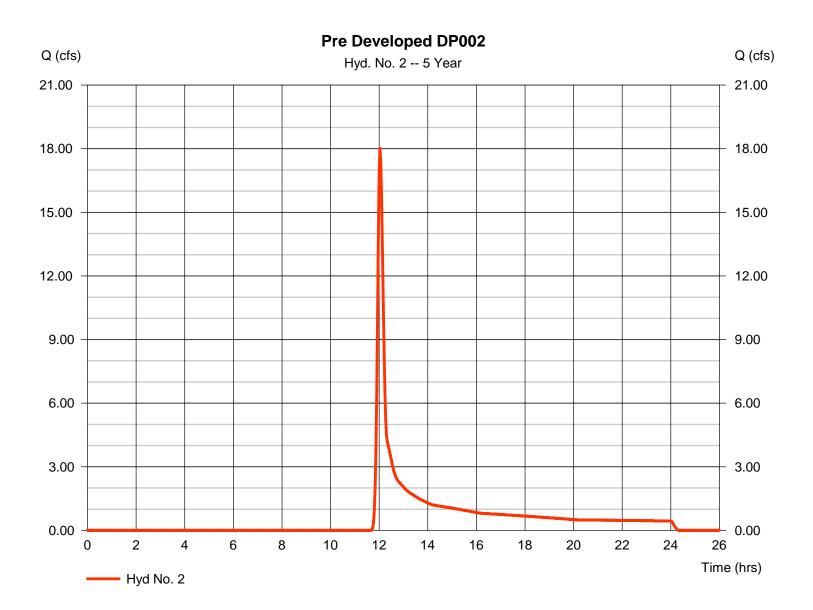
37

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 2

Pre Developed DP002

Hydrograph type	= SCS Runoff	Peak discharge	= 18.02 cfs
Storm frequency	= 5 yrs	Time to peak	= 12.03 hrs
Time interval	= 2 min	Hyd. volume	= 55,435 cuft
Drainage area	= 19.210 ac	Curve number	= 59.4
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= TR55	Time of conc. (Tc)	= 12.00 min
Total precip.	= 4.08 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

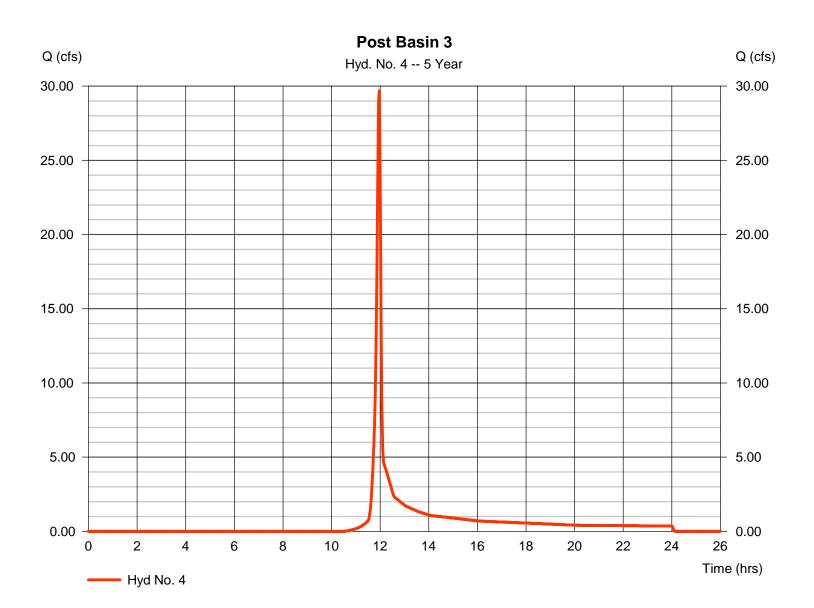


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 4

Post Basin 3

Hydrograph type	= SCS Runoff	Peak discharge	= 29.68 cfs
Storm frequency	= 5 yrs	Time to peak	= 11.97 hrs
Time interval	= 2 min	Hyd. volume	= 59,371 cuft
Drainage area	= 12.150 ac	Curve number	= 70.8
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 4.08 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484
Tc method Total precip.	= User = 4.08 in	Time of conc. (Tc) Distribution	= 5.00 min = Type II



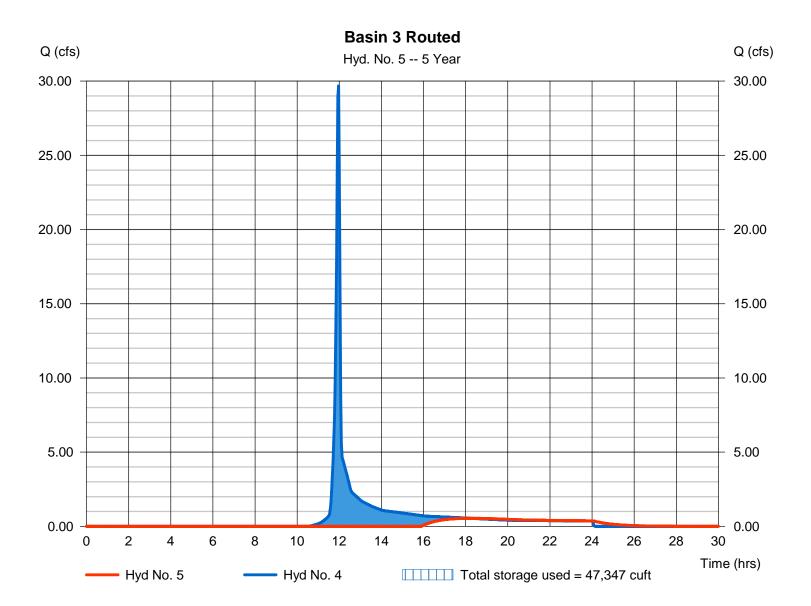
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 5

Basin 3 Routed

Hydrograph type	= Reservoir	Peak discharge	= 0.543 cfs
Storm frequency	= 5 yrs	Time to peak	= 18.23 hrs
Time interval	= 2 min	Hyd. volume	= 13,903 cuft
Inflow hyd. No.	= 4 - Post Basin 3	Max. Elevation	= 318.49 ft
Reservoir name	= Basin 3	Max. Storage	= 47,347 cuft

Storage Indication method used.

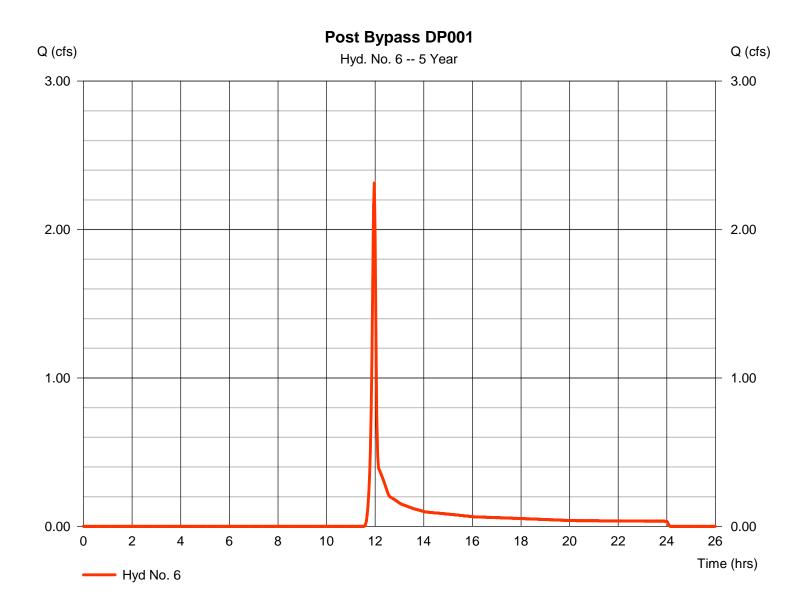


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 6

Post Bypass DP001

Hydrograph type	= SCS Runoff	Peak discharge	= 2.314 cfs
Storm frequency	= 5 yrs	Time to peak	= 11.97 hrs
Time interval	= 2 min	Hyd. volume	= 4,767 cuft
Drainage area	= 1.440 ac	Curve number	= 63.2
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 4.08 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



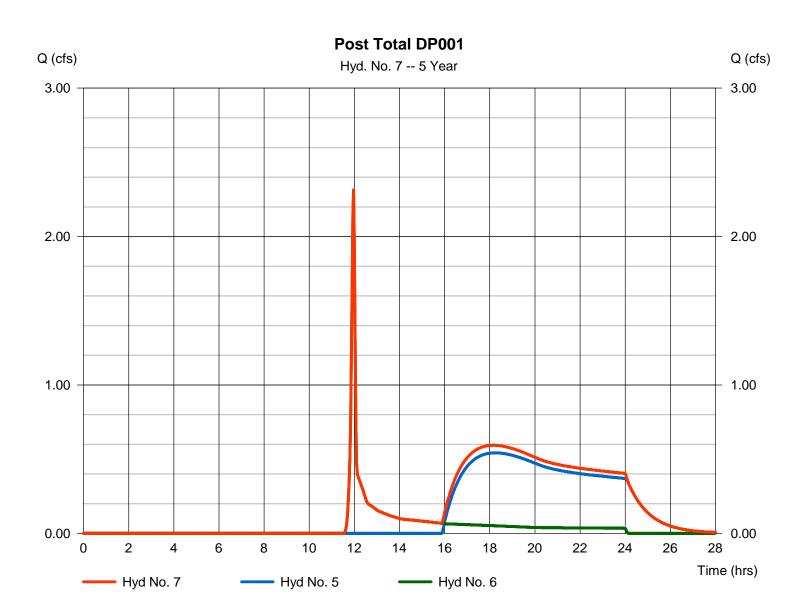
41

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 7

Post Total DP001

Hydrograph type	= Combine	Peak discharge	 = 2.314 cfs = 11.97 hrs = 18,670 cuft = 1.440 ac
Storm frequency	= 5 yrs	Time to peak	
Time interval	= 2 min	Hyd. volume	
Inflow hyds.	= 5, 6	Contrib. drain. area	
-			

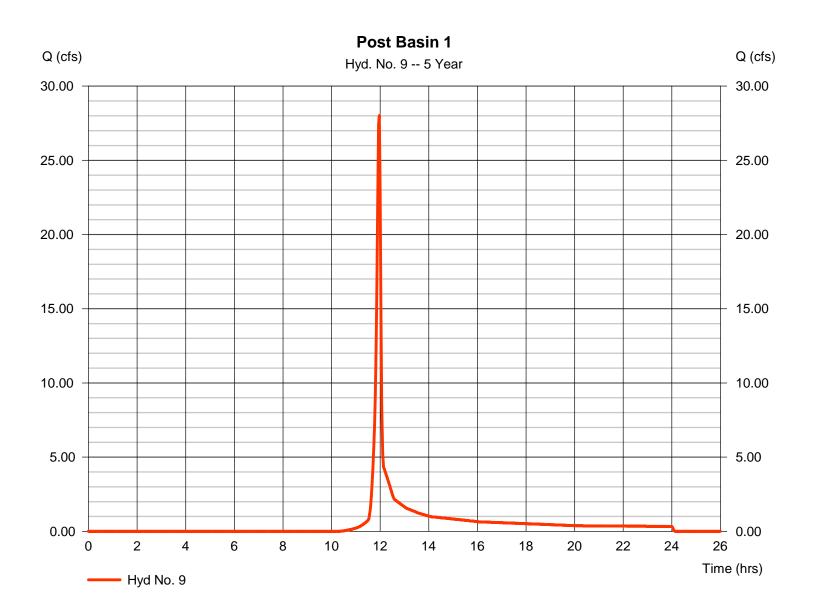


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 9

Post Basin 1

Hydrograph type	= SCS Runoff	Peak discharge	= 28.01 cfs
Storm frequency	= 5 yrs	Time to peak	= 11.97 hrs
Time interval	= 2 min	Hyd. volume	= 56,002 cuft
Drainage area	= 10.950 ac	Curve number	= 71.8
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 4.08 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



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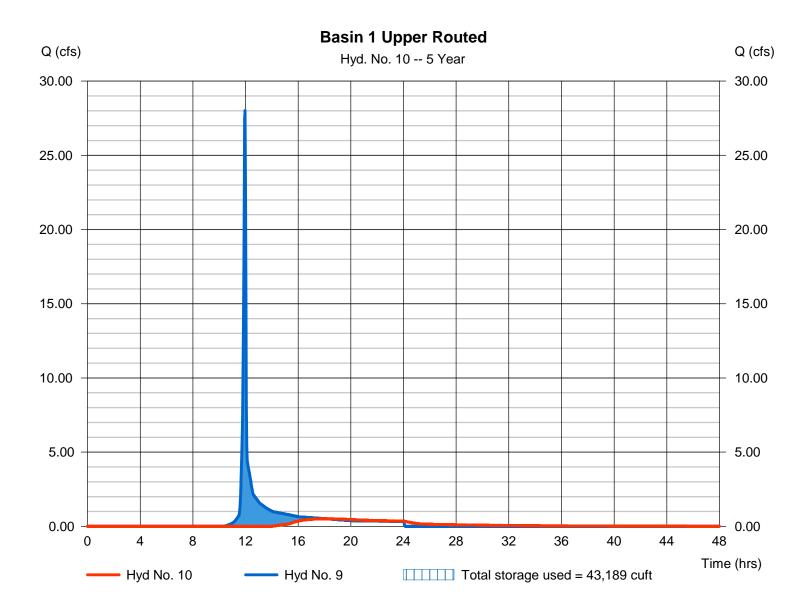
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 10

Basin 1 Upper Routed

Reservoir	Peak discharge	= 0.510 cfs
5 yrs	Time to peak	= 18.10 hrs
2 min	Hyd. volume	= 18,513 cuft
9 - Post Basin 1	Max. Elevation	= 322.71 ft
Basin 1 Upper	Max. Storage	= 43,189 cuft
=	5 yrs 2 min 9 - Post Basin 1	5 yrsTime to peak2 minHyd. volume9 - Post Basin 1Max. Elevation

Storage Indication method used.



44

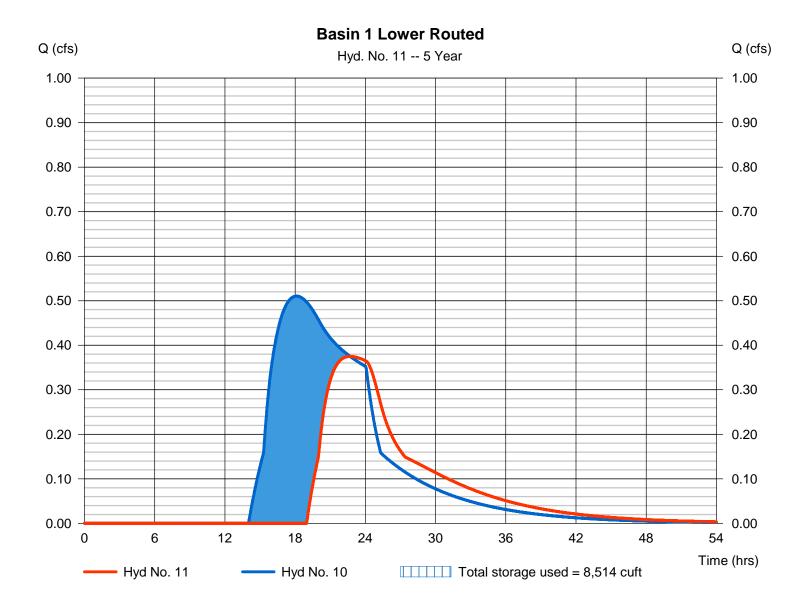
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 11

Basin 1 Lower Routed

Hydrograph type	= Reservoir	Peak discharge	= 0.375 cfs
Storm frequency	= 5 yrs	Time to peak	= 22.70 hrs
Time interval	= 2 min	Hyd. volume	= 12,317 cuft
Inflow hyd. No.	= 10 - Basin 1 Upper Routed	Max. Elevation	= 304.32 ft
Reservoir name	= Basin 1 Lower	Max. Storage	= 8,514 cuft
		maxi etorago	= 0,011 out

Storage Indication method used.



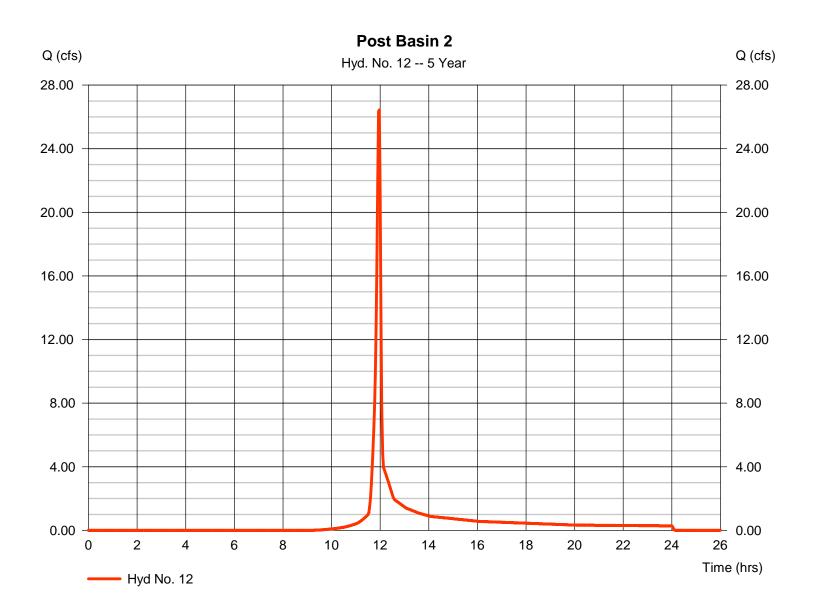
45

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 12

Post Basin 2

Hydrograph type	= SCS Runoff	Peak discharge	= 26.44 cfs
Storm frequency	= 5 yrs	Time to peak	= 11.97 hrs
Time interval	= 2 min	Hyd. volume	= 53,176 cuft
Drainage area	= 8.540 ac	Curve number	= 76.4
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 4.08 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



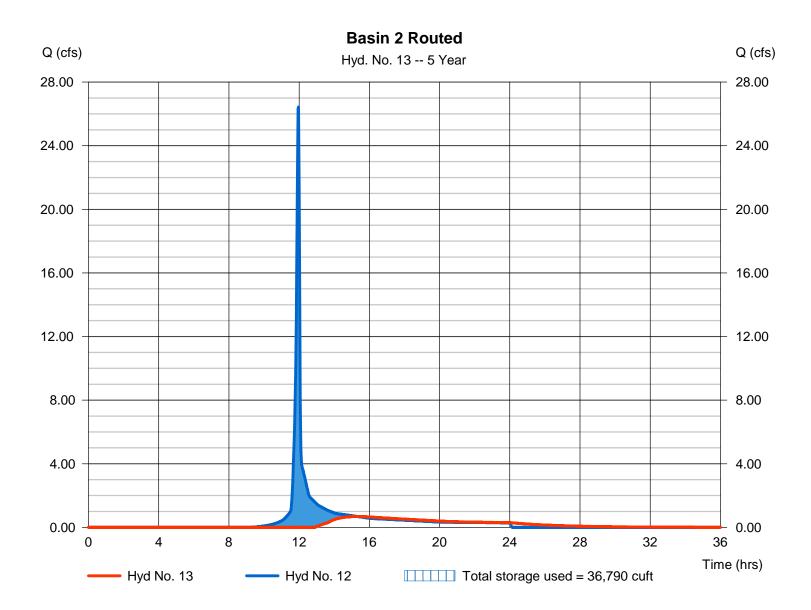
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 13

Basin 2 Routed

.682 cfs
5.30 hrs
0,822 cuft
09.11 ft
6,790 cuft
5 0 0

Storage Indication method used.

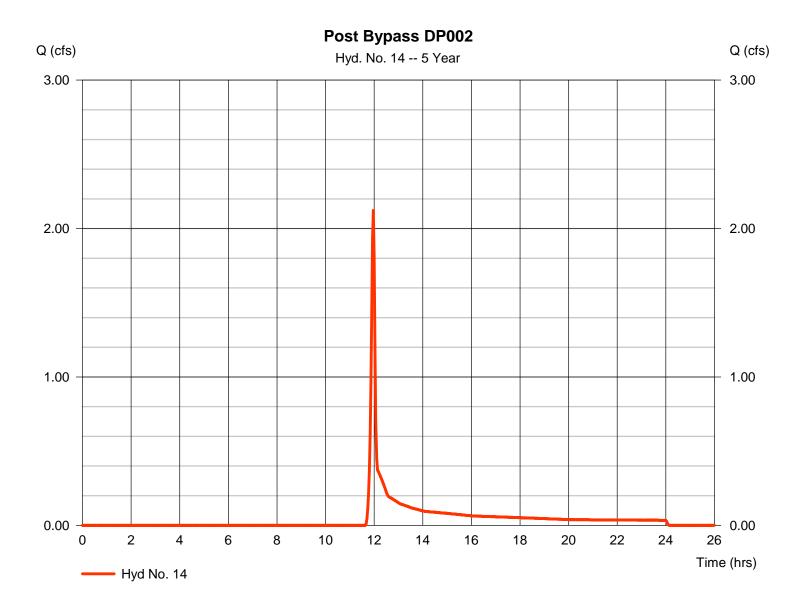


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 14

Post Bypass DP002

Hydrograph type	= SCS Runoff	Peak discharge	= 2.121 cfs
Storm frequency	= 5 yrs	Time to peak	= 11.97 hrs
Time interval	= 2 min	Hyd. volume	= 4,472 cuft
Drainage area	= 1.540 ac	Curve number	= 61
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 4.08 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

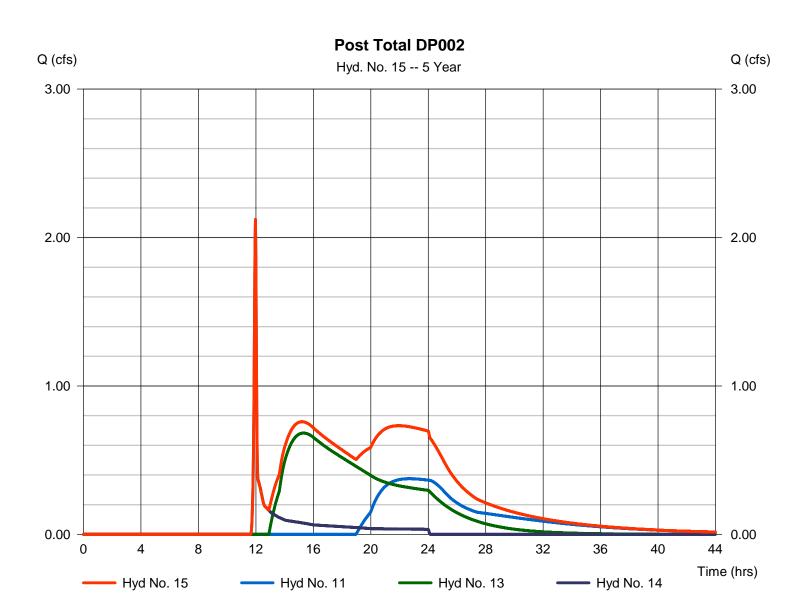


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 15

Post Total DP002

Storm frequency Time interval	= Combine = 5 yrs = 2 min = 11, 13, 14	Peak discharge Time to peak Hyd. volume Contrib. drain. area	 = 2.121 cfs = 11.97 hrs = 37,611 cuft = 1.540 ac
Innow Hyus.	- 11, 13, 14	Contrib. urain. area	= 1.540 ac



Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

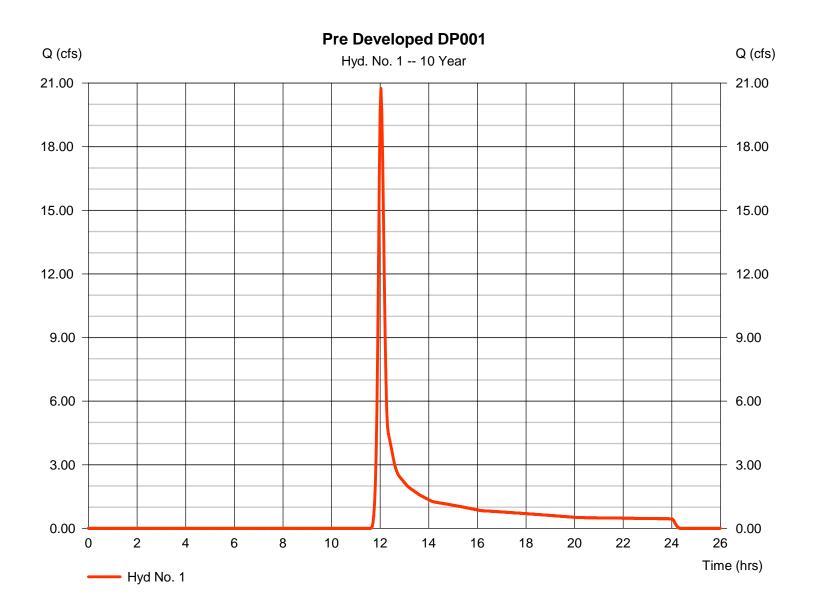
Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	20.75	2	722	60,334				Pre Developed DP001
2	SCS Runoff	28.64	2	722	81,339				Pre Developed DP002
4	SCS Runoff	39.83	2	718	79,751				Post Basin 3
5	Reservoir	1.424	2	838	34,284	4	318.61	49,983	Basin 3 Routed
6	SCS Runoff	3.349	2	718	6,757				Post Bypass DP001
7	Combine	3.349	2	718	41,040	5, 6			Post Total DP001
9	SCS Runoff	37.29	2	718	74,773				Post Basin 1
10	Reservoir	1.244	2	848	37,284	9	322.89	46,611	Basin 1 Upper Routed
11	Reservoir	0.928	2	996	31,087	10	304.57	10,321	Basin 1 Lower Routed
12	SCS Runoff	34.26	2	716	69,196				Post Basin 2
13	Reservoir	1.706	2	788	36,842	12	309.33	40,115	Basin 2 Routed
14	SCS Runoff	3.170	2	718	6,463				Post Bypass DP002
15	Combine	3.170	2	718	74,392	11, 13, 14			Post Total DP002
SW	/M.gpw				Return	Period: 10 Y	/ear	Thursday,	09 / 30 / 2021

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 1

Pre Developed DP001

Hydrograph type	= SCS Runoff	Peak discharge	= 20.75 cfs
Storm frequency	= 10 yrs	Time to peak	= 12.03 hrs
Time interval	= 2 min	Hyd. volume	= 60,334 cuft
Drainage area	= 15.430 ac	Curve number	= 58
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= TR55	Time of conc. (Tc)	= 12.00 min
Total precip.	= 4.77 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



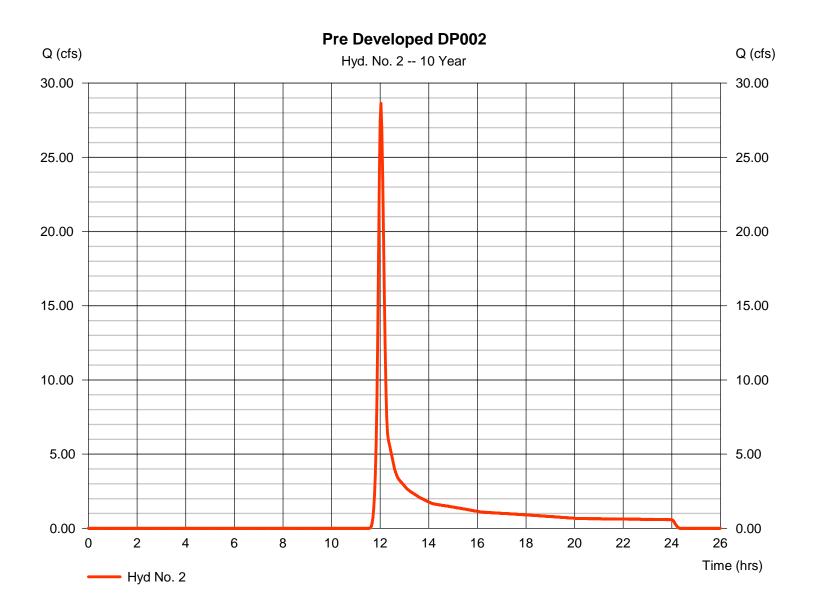
51

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 2

Pre Developed DP002

Hydrograph type	= SCS Runoff	Peak discharge	= 28.64 cfs
Storm frequency	= 10 yrs	Time to peak	= 12.03 hrs
Time interval	= 2 min	Hyd. volume	= 81,339 cuft
Drainage area	= 19.210 ac	Curve number	= 59.4
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= TR55	Time of conc. (Tc)	= 12.00 min
Total precip.	= 4.77 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



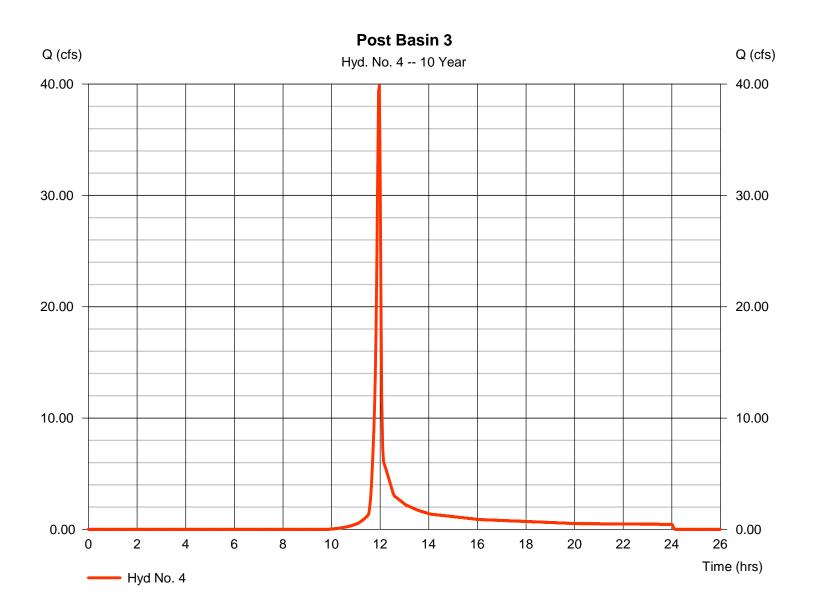
52

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 4

Post Basin 3

Hydrograph type	= SCS Runoff	Peak discharge	= 39.83 cfs
Storm frequency	= 10 yrs	Time to peak	= 11.97 hrs
Time interval	= 2 min	Hyd. volume	= 79,751 cuft
Drainage area	= 12.150 ac	Curve number	= 70.8
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 4.77 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



53

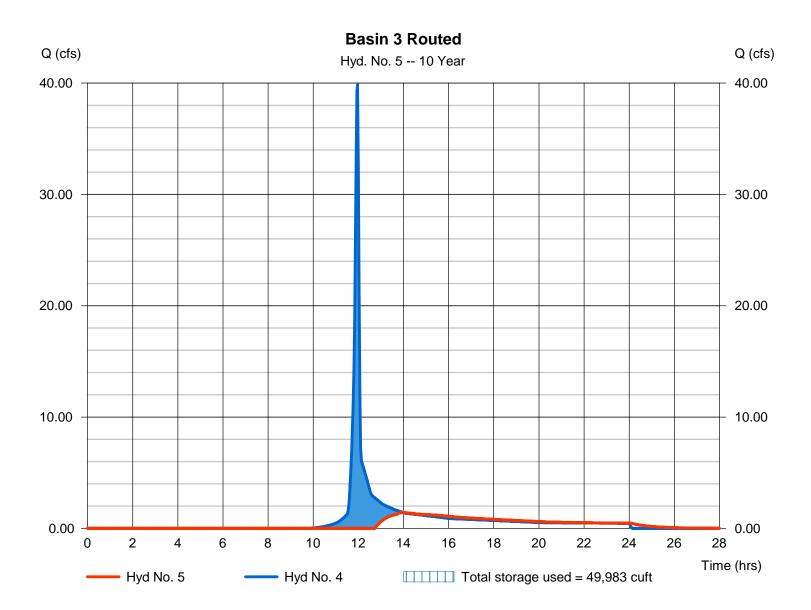
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 5

Basin 3 Routed

Hydrograph type	= Reservoir	Peak discharge	= 1.424 cfs
Storm frequency	= 10 yrs	Time to peak	= 13.97 hrs
Time interval	= 2 min	Hyd. volume	= 34,284 cuft
Inflow hyd. No.	= 4 - Post Basin 3	Max. Elevation	= 318.61 ft
Reservoir name	= Basin 3	Max. Storage	= 49,983 cuft

Storage Indication method used.

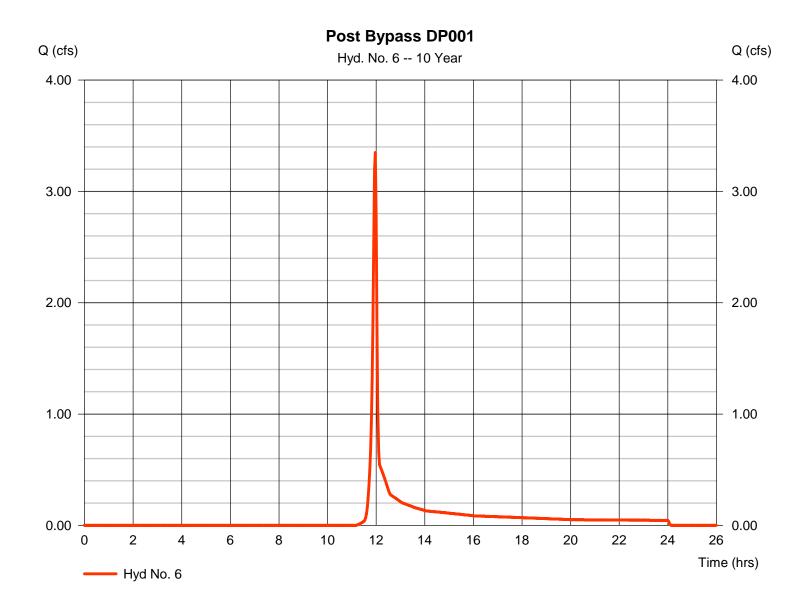


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 6

Post Bypass DP001

Hydrograph type	= SCS Runoff	Peak discharge	= 3.349 cfs
Storm frequency	= 10 yrs	Time to peak	= 11.97 hrs
Time interval	= 2 min	Hyd. volume	= 6,757 cuft
Drainage area	= 1.440 ac	Curve number	= 63.2
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 4.77 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

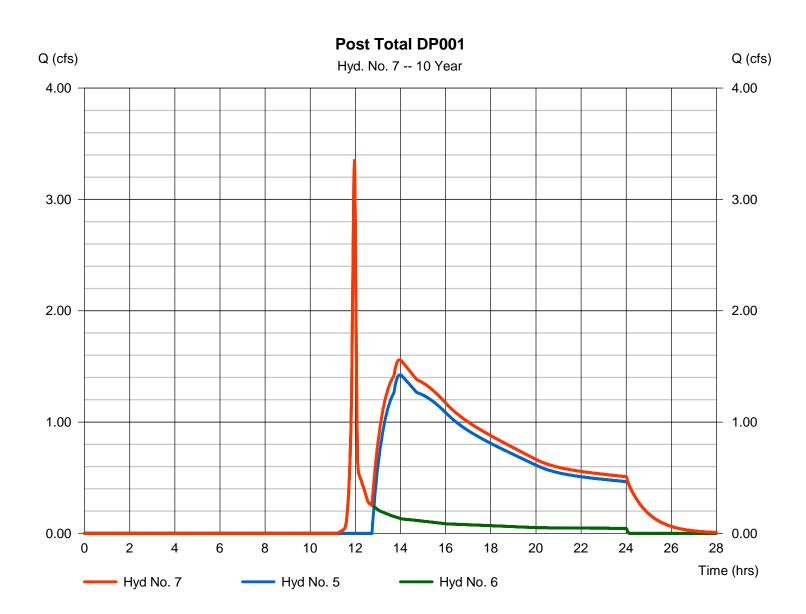


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 7

Post Total DP001

Hydrograph type	= Combine	Peak discharge	= 3.349 cfs
Storm frequency	= 10 yrs	Time to peak	= 11.97 hrs
Time interval	= 2 min	Hyd. volume	= 41,040 cuft
Inflow hyds.	= 5, 6	Contrib. drain. area	= 1.440 ac



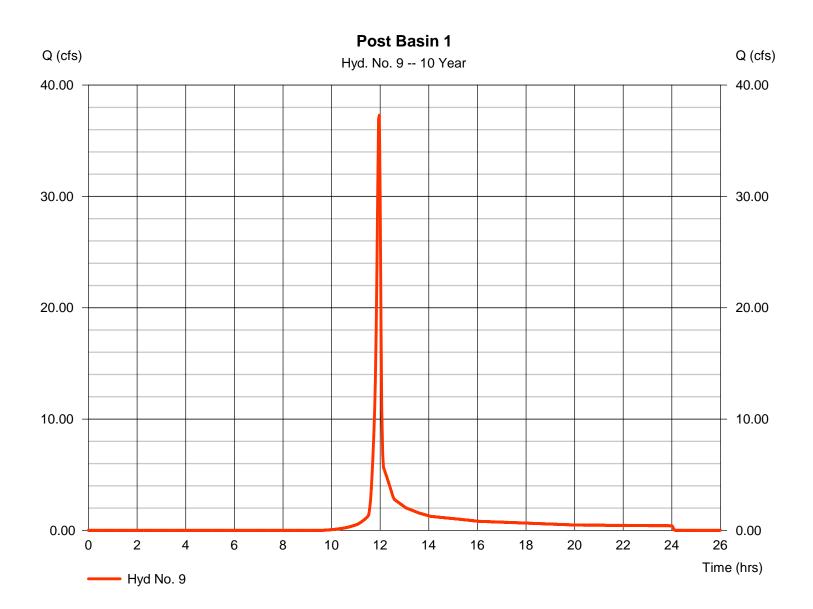
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Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 9

Post Basin 1

Hydrograph type	= SCS Runoff	Peak discharge	= 37.29 cfs
Storm frequency	= 10 yrs	Time to peak	= 11.97 hrs
Time interval	= 2 min	Hyd. volume	= 74,773 cuft
Drainage area	= 10.950 ac	Curve number	= 71.8
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 4.77 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



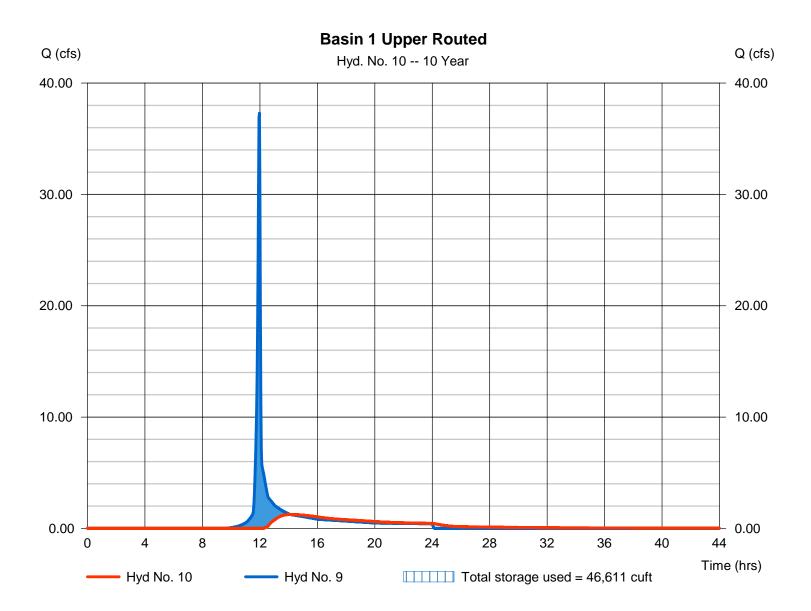
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 10

Basin 1 Upper Routed

Hydrograph type	= Reservoir	Peak discharge	= 1.244 cfs
Storm frequency	= 10 yrs	Time to peak	= 14.13 hrs
Time interval	= 2 min	Hyd. volume	= 37,284 cuft
Inflow hyd. No.	= 9 - Post Basin 1	Max. Elevation	= 322.89 ft
Reservoir name	= Basin 1 Upper	Max. Storage	= 46,611 cuft

Storage Indication method used.



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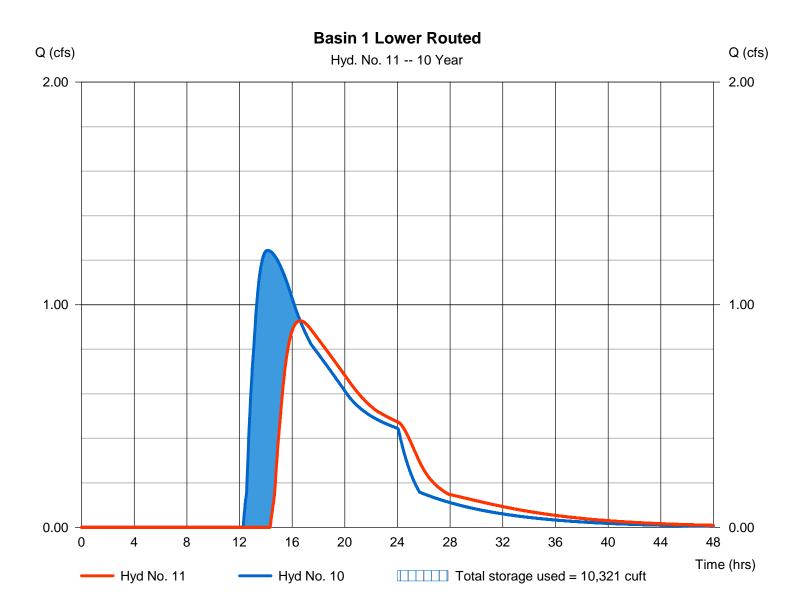
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 11

Basin 1 Lower Routed

= Reservoir	Peak discharge	= 0.928 cfs
= 10 yrs	Time to peak	= 16.60 hrs
= 2 min	Hyd. volume	= 31,087 cuft
= 10 - Basin 1 Upper Routed	Max. Elevation	= 304.57 ft
= Basin 1 Lower	Max. Storage	= 10,321 cuft
	= 10 yrs= 2 min= 10 - Basin 1 Upper Routed	= 10 yrsTime to peak= 2 minHyd. volume= 10 - Basin 1 Upper RoutedMax. Elevation

Storage Indication method used.

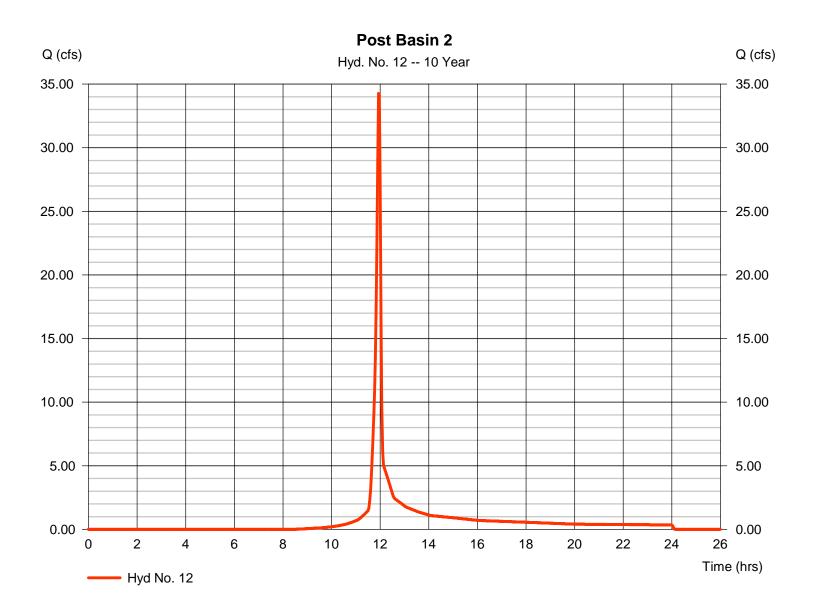


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 12

Post Basin 2

Hydrograph type	= SCS Runoff	Peak discharge	= 34.26 cfs
Storm frequency	= 10 yrs	Time to peak	= 11.93 hrs
Time interval	= 2 min	Hyd. volume	= 69,196 cuft
Drainage area	= 8.540 ac	Curve number	= 76.4
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 4.77 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



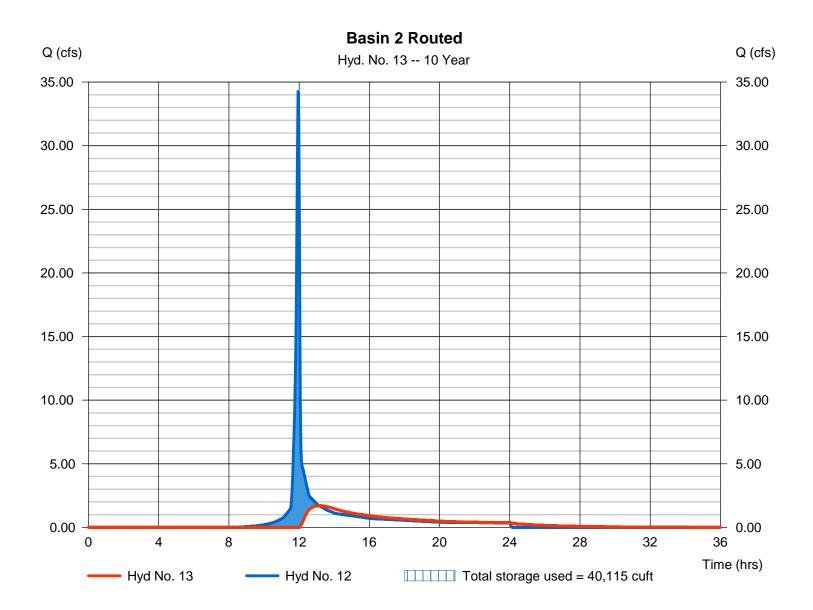
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 13

Basin 2 Routed

Hydrograph type	= Reservoir	Peak discharge	= 1.706 cfs
Storm frequency	= 10 yrs	Time to peak	= 13.13 hrs
Time interval	= 2 min	Hyd. volume	= 36,842 cuft
Inflow hyd. No.	= 12 - Post Basin 2	Max. Elevation	= 309.33 ft
Reservoir name	= Basin 2	Max. Storage	= 40,115 cuft

Storage Indication method used.

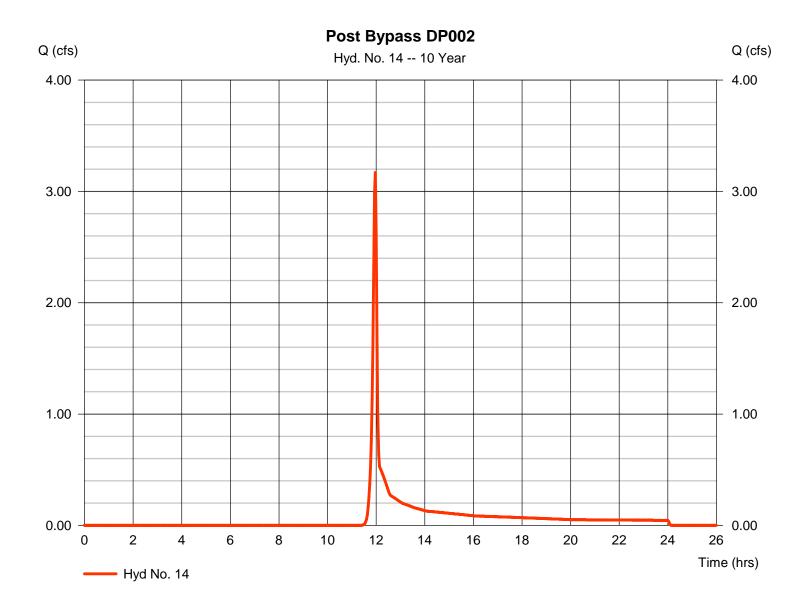


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 14

Post Bypass DP002

Hydrograph type	= SCS Runoff	Peak discharge	= 3.170 cfs
Storm frequency	= 10 yrs	Time to peak	= 11.97 hrs
Time interval	= 2 min	Hyd. volume	= 6,463 cuft
Drainage area	= 1.540 ac	Curve number	= 61
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 4.77 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

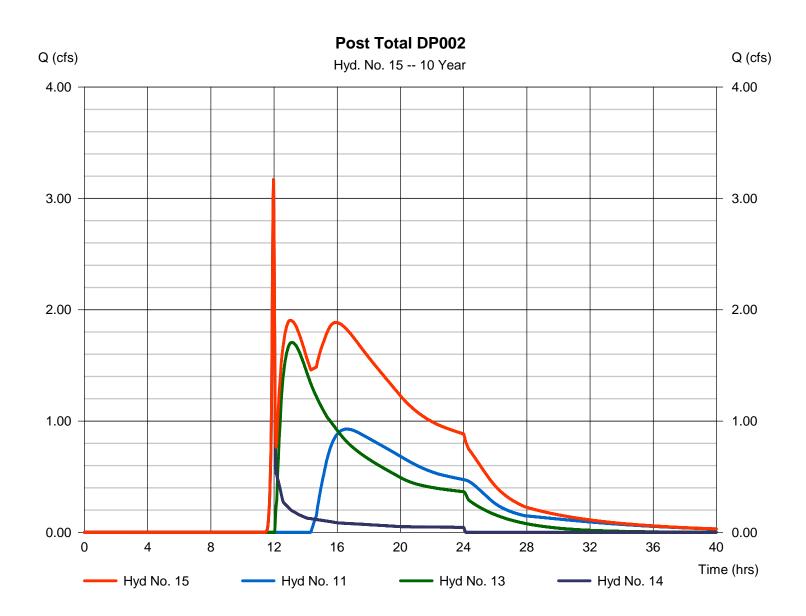


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 15

Post Total DP002

Hydrograph type	= Combine	Peak discharge	= 3.170 cfs
Storm frequency	= 10 yrs	Time to peak	= 11.97 hrs
Time interval	= 2 min	Hyd. volume	= 74,392 cuft
Inflow hyds.	= 11, 13, 14	Contrib. drain. area	= 1.540 ac



Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

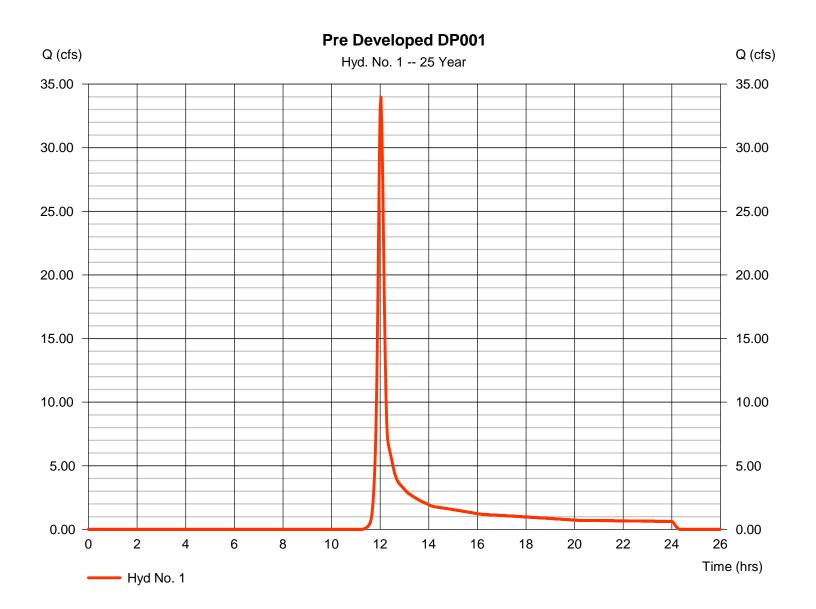
Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	33.98	2	722	92,948				Pre Developed DP001
2	SCS Runoff	45.70	2	722	123,600				Pre Developed DP002
4	SCS Runoff	55.13	2	718	111,161				Post Basin 3
5	Reservoir	6.253	2	740	65,693	4	318.79	53,978	Basin 3 Routed
6	SCS Runoff	4.966	2	718	9,933				Post Bypass DP001
7	Combine	6.859	2	740	75,627	5, 6			Post Total DP001
9	SCS Runoff	51.31	2	716	103,585				Post Basin 1
10	Reservoir	3.633	2	756	66,096	9	323.31	54,387	Basin 1 Upper Routed
11	Reservoir	2.071	2	874	59,899	10	305.33	15,802	Basin 1 Lower Routed
12	SCS Runoff	46.02	2	716	93,362				Post Basin 2
13	Reservoir	5.509	2	736	61,008	12	309.83	47,406	Basin 2 Routed
14	SCS Runoff	4.826	2	718	9,678				Post Bypass DP002
15	Combine	6.169	2	736	130,585	11, 13, 14			Post Total DP002
SW	M.gpw				Return F	Period: 25 Y	/ear	Thursday,	09 / 30 / 2021

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 1

Pre Developed DP001

Hydrograph type	= SCS Runoff	Peak discharge	= 33.98 cfs
Storm frequency	= 25 yrs	Time to peak	= 12.03 hrs
Time interval	= 2 min	Hyd. volume	= 92,948 cuft
Drainage area	= 15.430 ac	Curve number	= 58
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= TR55	Time of conc. (Tc)	= 12.00 min
Total precip.	= 5.76 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

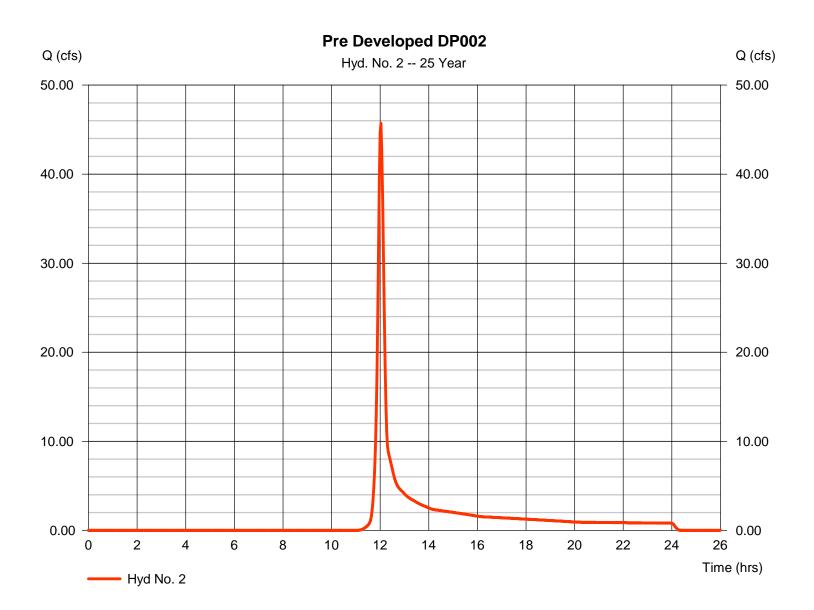


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 2

Pre Developed DP002

Hydrograph type	= SCS Runoff	Peak discharge	= 45.70 cfs
Storm frequency	= 25 yrs	Time to peak	= 12.03 hrs
Time interval	= 2 min	Hyd. volume	= 123,600 cuft
Drainage area	= 19.210 ac	Curve number	= 59.4
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= TR55	Time of conc. (Tc)	= 12.00 min
Total precip.	= 5.76 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

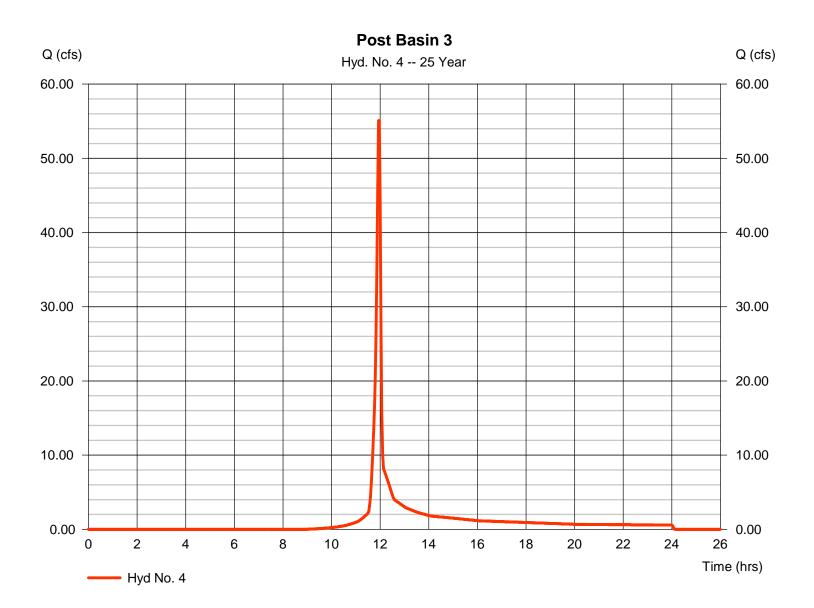


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 4

Post Basin 3

Hydrograph type	= SCS Runoff	Peak discharge	= 55.13 cfs
Storm frequency	= 25 yrs	Time to peak	= 11.97 hrs
Time interval	= 2 min	Hyd. volume	= 111,161 cuft
Drainage area	= 12.150 ac	Curve number	= 70.8
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 5.76 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



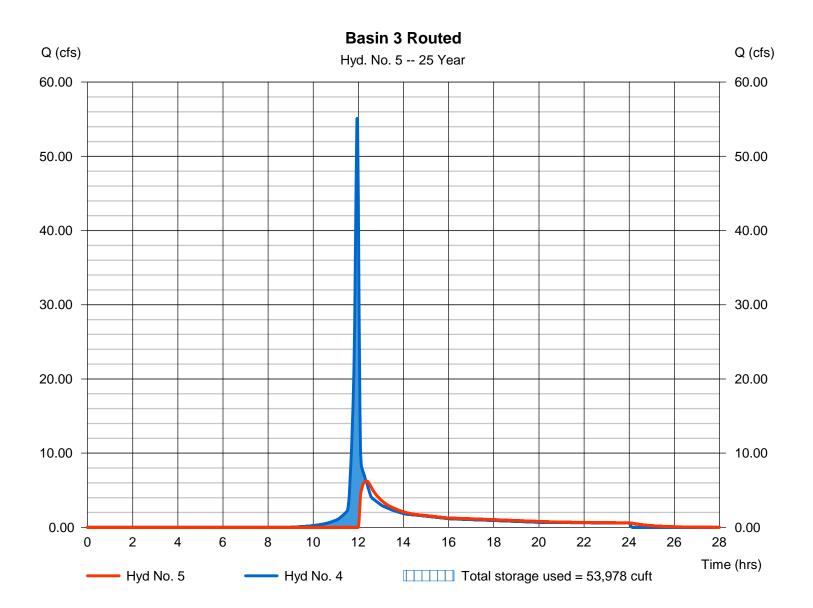
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 5

Basin 3 Routed

Hydrograph type	= Reservoir	Peak discharge	= 6.253 cfs
Storm frequency	= 25 yrs	Time to peak	= 12.33 hrs
Time interval	= 2 min	Hyd. volume	= 65,693 cuft
Inflow hyd. No.	= 4 - Post Basin 3	Max. Elevation	= 318.79 ft
Reservoir name	= Basin 3	Max. Storage	= 53,978 cuft

Storage Indication method used.



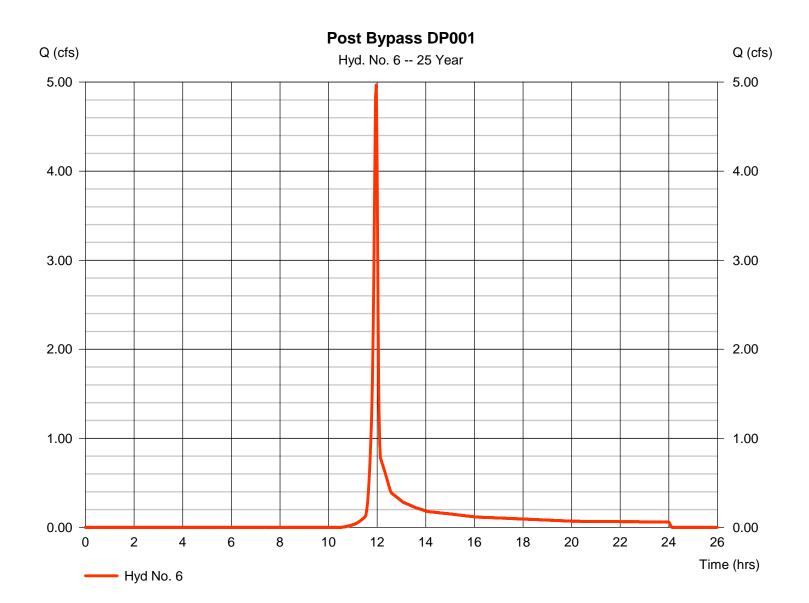
68

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 6

Post Bypass DP001

Hydrograph type	= SCS Runoff	Peak discharge	= 4.966 cfs
Storm frequency	= 25 yrs	Time to peak	= 11.97 hrs
Time interval	= 2 min	Hyd. volume	= 9,933 cuft
Drainage area	= 1.440 ac	Curve number	= 63.2
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 5.76 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Thursday, 09 / 30 / 2021

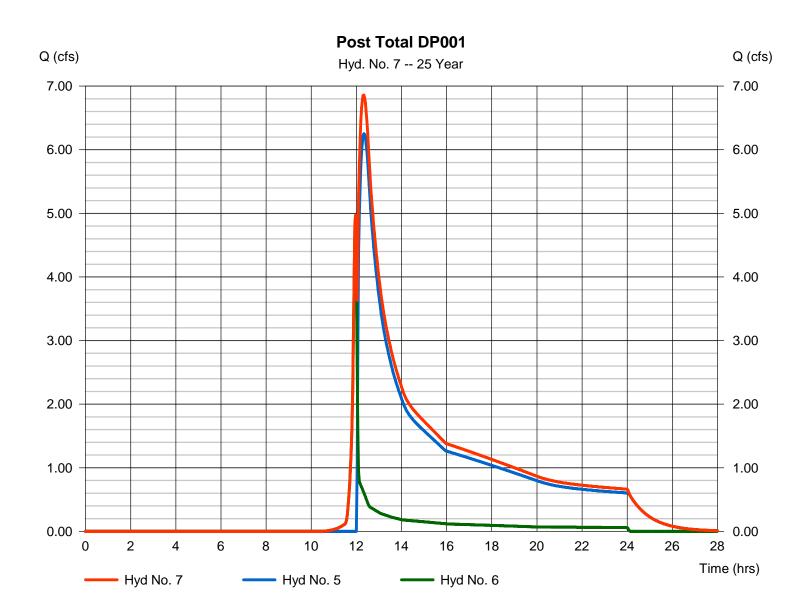
69

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 7

Post Total DP001

Hydrograph type	= Combine	Peak discharge	= 6.859 cfs
Storm frequency	= 25 yrs	Time to peak	= 12.33 hrs
Time interval	= 2 min	Hyd. volume	= 75,627 cuft
Inflow hyds.	= 5, 6	Contrib. drain. area	= 1.440 ac

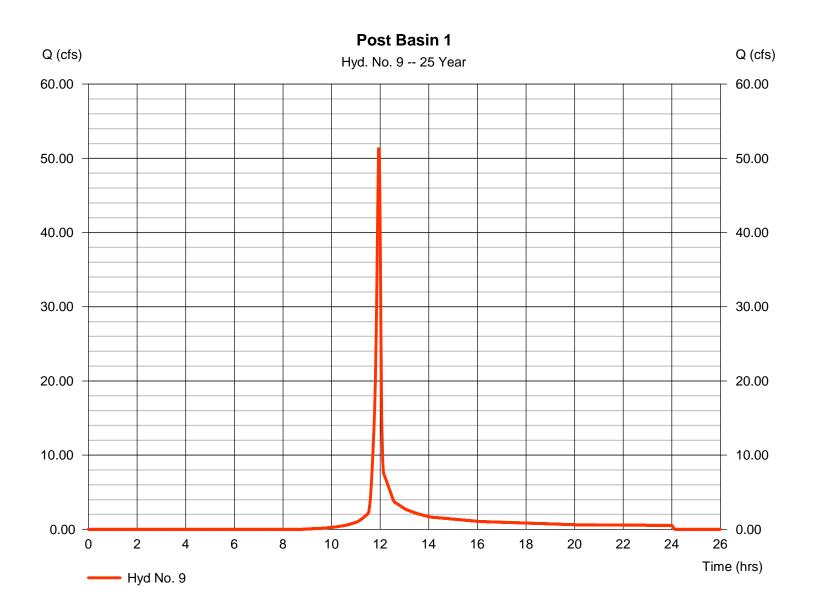


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 9

Post Basin 1

Hydrograph type	= SCS Runoff	Peak discharge	= 51.31 cfs
Storm frequency	= 25 yrs	Time to peak	= 11.93 hrs
Time interval	= 2 min	Hyd. volume	= 103,585 cuft
Drainage area	= 10.950 ac	Curve number	= 71.8
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 5.76 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



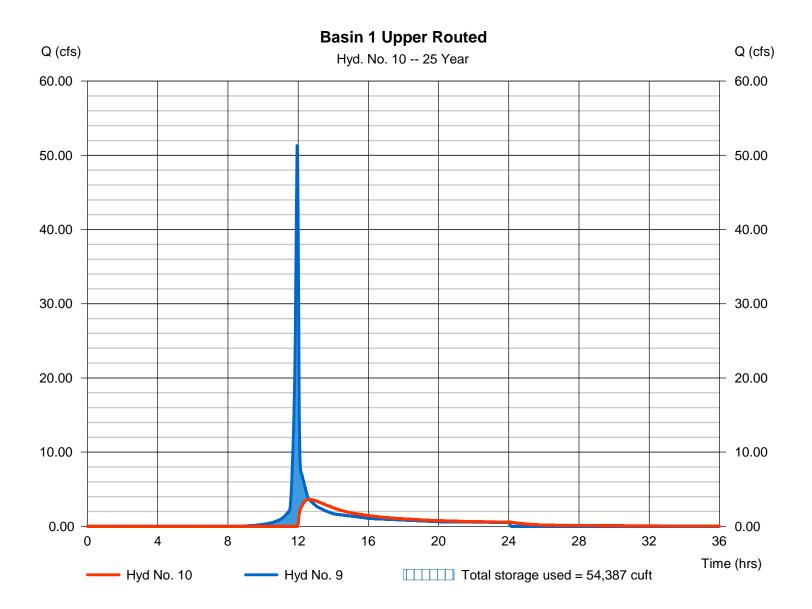
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 10

Basin 1 Upper Routed

Hydrograph type	= Reservoir	Peak discharge	= 3.633 cfs
Storm frequency	= 25 yrs	Time to peak	= 12.60 hrs
Time interval	= 2 min	Hyd. volume	= 66,096 cuft
Inflow hyd. No.	= 9 - Post Basin 1	Max. Elevation	= 323.31 ft
Reservoir name	= Basin 1 Upper	Max. Storage	= 54,387 cuft

Storage Indication method used.



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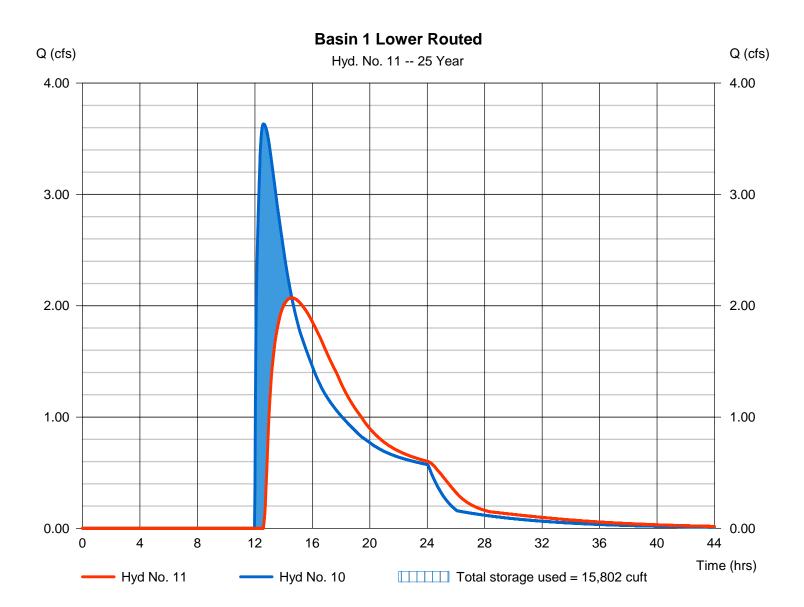
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 11

Basin 1 Lower Routed

Hydrograph type	= Reservoir	Peak discharge	= 2.071 cfs
Storm frequency	= 25 yrs	Time to peak	= 14.57 hrs
Time interval	= 2 min	Hyd. volume	= 59,899 cuft
Inflow hyd. No.	= 10 - Basin 1 Upper Routed	Max. Elevation	= 305.33 ft
Reservoir name	= Basin 1 Lower	Max. Storage	= 15,802 cuft

Storage Indication method used.

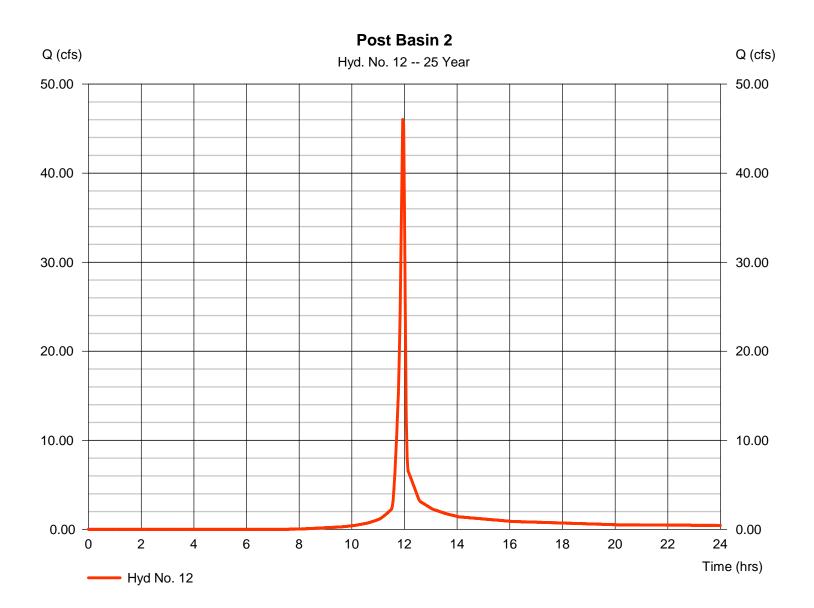


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 12

Post Basin 2

Hydrograph type	= SCS Runoff	Peak discharge	= 46.02 cfs
Storm frequency	= 25 yrs	Time to peak	= 11.93 hrs
Time interval	= 2 min	Hyd. volume	= 93,362 cuft
Drainage area	= 8.540 ac	Curve number	= 76.4
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 5.76 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484
		-	



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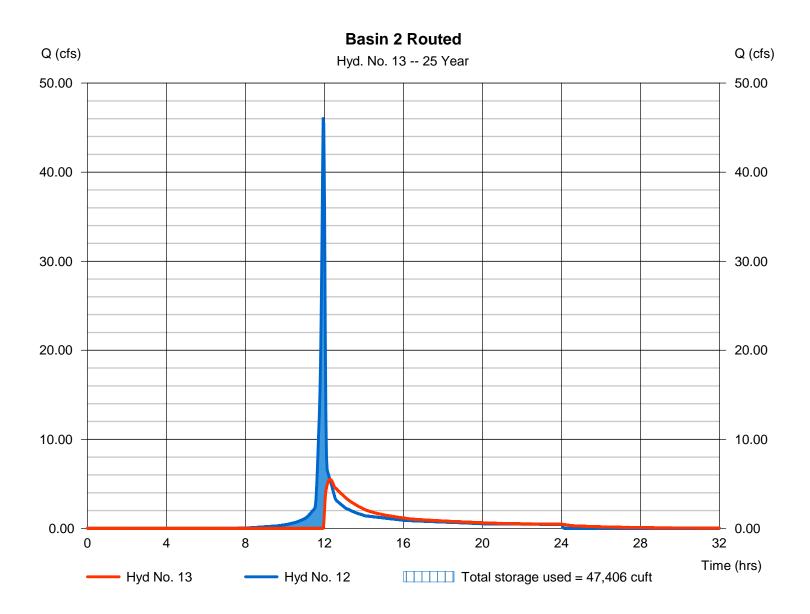
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 13

Basin 2 Routed

Hydrograph type	= Reservoir	Peak discharge	= 5.509 cfs
Storm frequency	= 25 yrs	Time to peak	= 12.27 hrs
Time interval	= 2 min	Hyd. volume	= 61,008 cuft
Inflow hyd. No.	= 12 - Post Basin 2	Max. Elevation	= 309.83 ft
Reservoir name	= Basin 2	Max. Storage	= 47,406 cuft

Storage Indication method used.

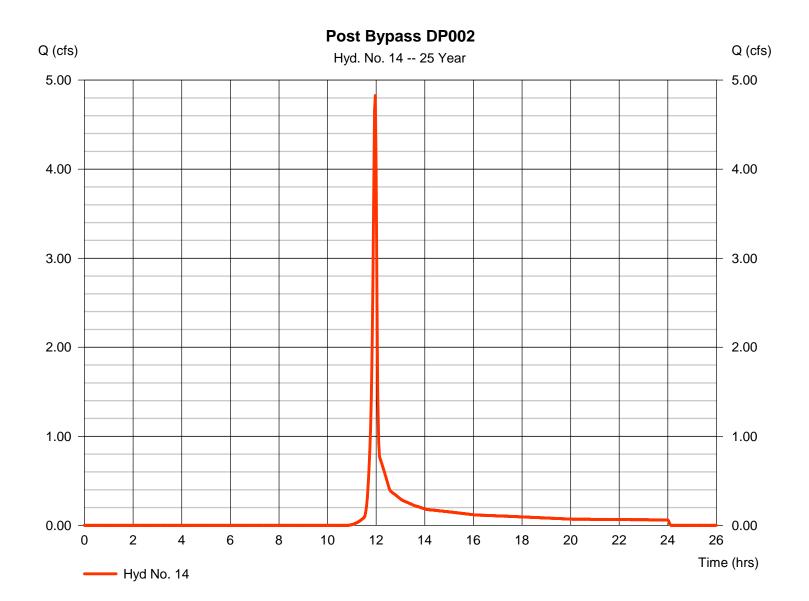


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 14

Post Bypass DP002

Hydrograph type	= SCS Runoff	Peak discharge	= 4.826 cfs
Storm frequency	= 25 yrs	Time to peak	= 11.97 hrs
Time interval	= 2 min	Hyd. volume	= 9,678 cuft
Drainage area	= 1.540 ac	Curve number	= 61
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 5.76 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

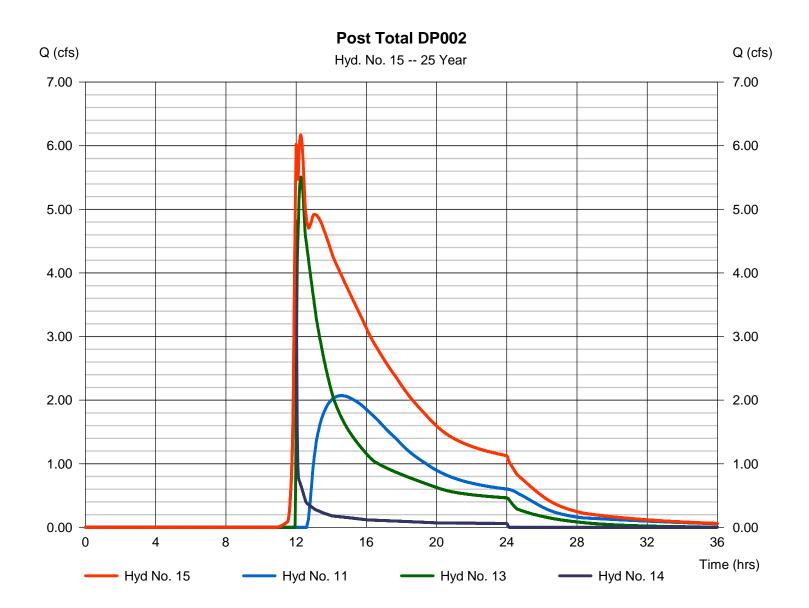


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 15

Post Total DP002

Hydrograph type	= Combine	Peak discharge	= 6.169 cfs
Storm frequency	= 25 yrs	Time to peak	= 12.27 hrs
Time interval	= 2 min	Hyd. volume	= 130,585 cuft
Inflow hyds.	= 11, 13, 14	Contrib. drain. area	= 1.540 ac



Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

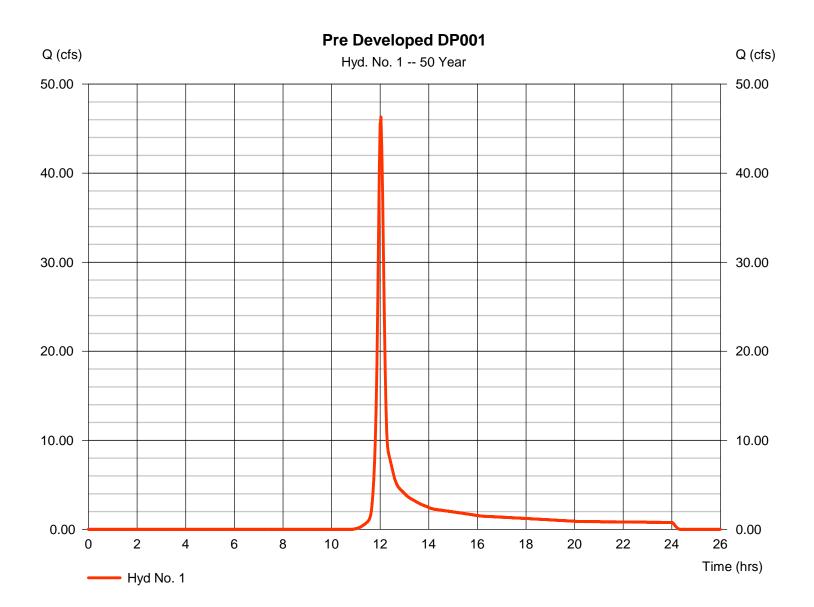
Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	46.28	2	722	123,698				Pre Developed DP001
2	SCS Runoff	61.46	2	722	163,179				Pre Developed DP002
4	SCS Runoff	68.94	2	716	139,306				Post Basin 3
5	Reservoir	19.64	2	724	93,838	4	319.16	62,202	Basin 3 Routed
6	SCS Runoff	6.425	2	718	12,860				Post Bypass DP001
7	Combine	21.53	2	724	106,698	5, 6			Post Total DP001
9	SCS Runoff	63.94	2	716	129,319				Post Basin 1
10	Reservoir	10.69	2	726	91,830	9	323.71	61,860	Basin 1 Upper Routed
11	Reservoir	5.226	2	772	85,633	10	305.66	18,217	Basin 1 Lower Routed
12	SCS Runoff	56.20	2	716	114,655				Post Basin 2
13	Reservoir	18.98	2	724	82,300	12	310.22	53,973	Basin 2 Routed
14	SCS Runoff	6.335	2	718	12,668				Post Bypass DP002
15	Combine	21.32	2	722	180,602	11, 13, 14			Post Total DP002
SW	M.gpw				Return F	Period: 50 Y	/ear	Thursday,	09 / 30 / 2021

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 1

Pre Developed DP001

Hydrograph type	= SCS Runoff	Peak discharge	= 46.28 cfs
Storm frequency	= 50 yrs	Time to peak	= 12.03 hrs
Time interval	= 2 min	Hyd. volume	= 123,698 cuft
Drainage area	= 15.430 ac	Curve number	= 58
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= TR55	Time of conc. (Tc)	= 12.00 min
Total precip.	= 6.60 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



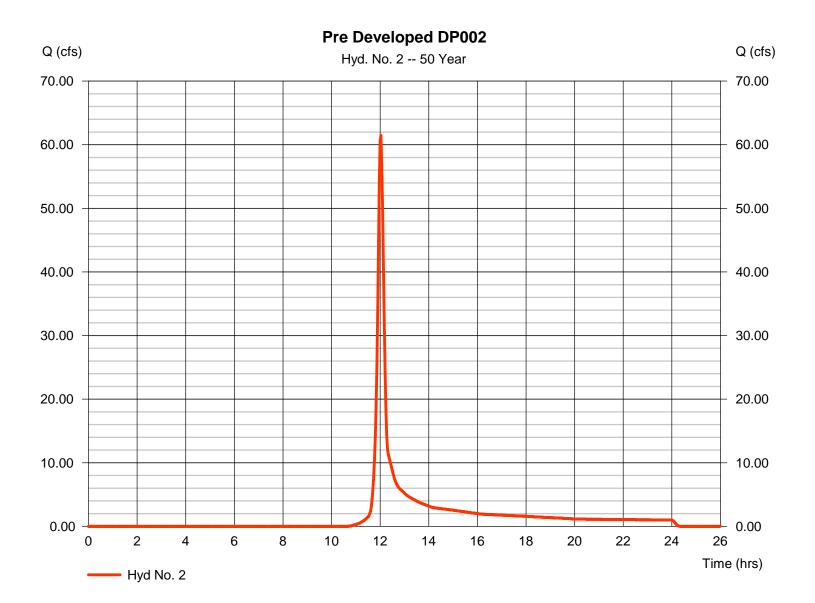
79

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 2

Pre Developed DP002

Hydrograph type	= SCS Runoff	Peak discharge	= 61.46 cfs
Storm frequency	= 50 yrs	Time to peak	= 12.03 hrs
Time interval	= 2 min	Hyd. volume	= 163,179 cuft
Drainage area	= 19.210 ac	Curve number	= 59.4
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= TR55	Time of conc. (Tc)	= 12.00 min
Total precip.	= 6.60 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

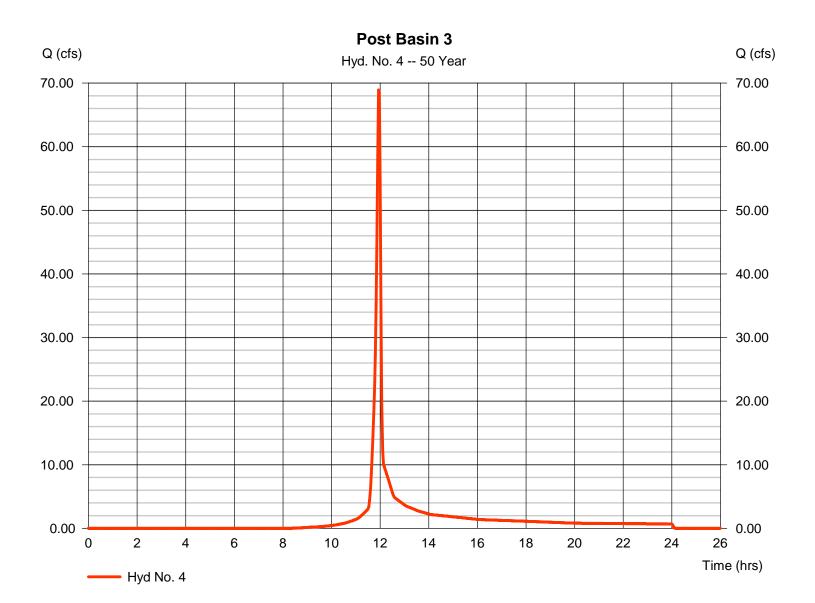


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 4

Post Basin 3

Hydrograph type	= SCS Runoff	Peak discharge	= 68.94 cfs
Storm frequency	= 50 yrs	Time to peak	= 11.93 hrs
Time interval	= 2 min	Hyd. volume	= 139,306 cuft
Drainage area	= 12.150 ac	Curve number	= 70.8
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 6.60 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



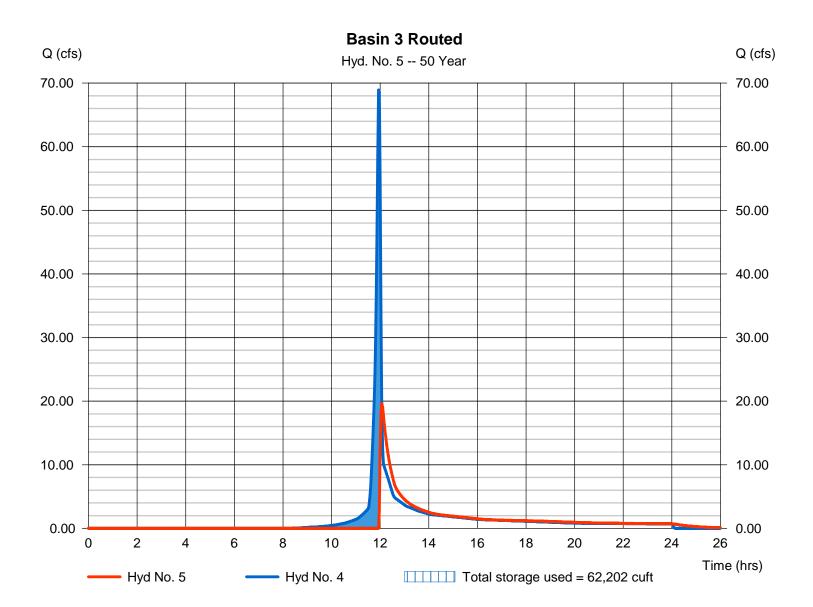
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 5

Basin 3 Routed

Hydrograph type	= Reservoir	Peak discharge	= 19.64 cfs
Storm frequency	= 50 yrs	Time to peak	= 12.07 hrs
Time interval	= 2 min	Hyd. volume	= 93,838 cuft
Inflow hyd. No.	= 4 - Post Basin 3	Max. Elevation	= 319.16 ft
Reservoir name	= Basin 3	Max. Storage	= 62,202 cuft

Storage Indication method used.

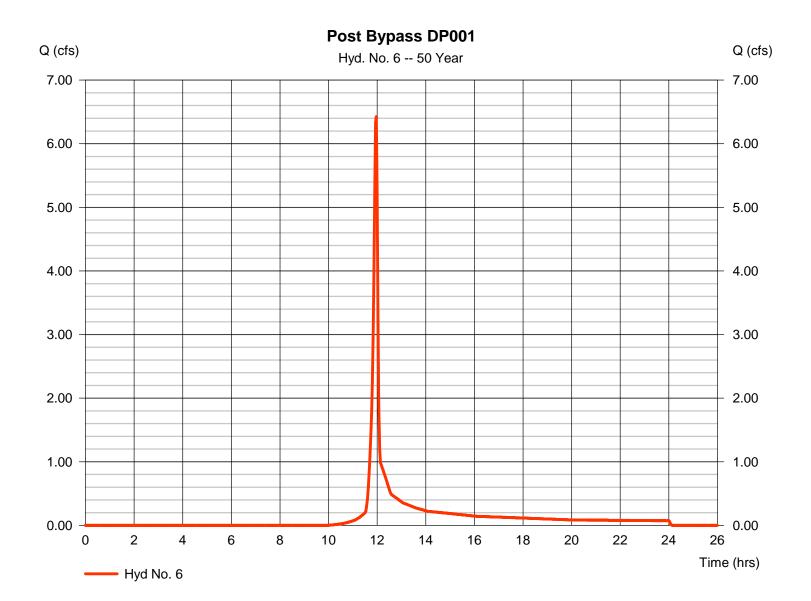


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 6

Post Bypass DP001

Hydrograph type	= SCS Runoff	Peak discharge	= 6.425 cfs
Storm frequency	= 50 yrs	Time to peak	= 11.97 hrs
Time interval	= 2 min	Hyd. volume	= 12,860 cuft
Drainage area	= 1.440 ac	Curve number	= 63.2
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 6.60 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

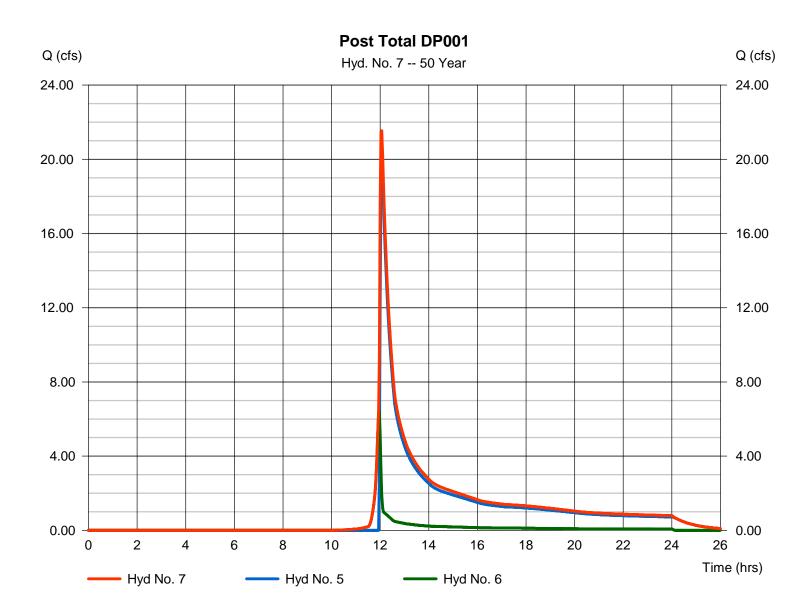


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 7

Post Total DP001

Hydrograph type	= Combine	Peak discharge	 = 21.53 cfs = 12.07 hrs = 106,698 cuft = 1.440 ac
Storm frequency	= 50 yrs	Time to peak	
Time interval	= 2 min	Hyd. volume	
Inflow hyds.	= 5, 6	Contrib. drain. area	



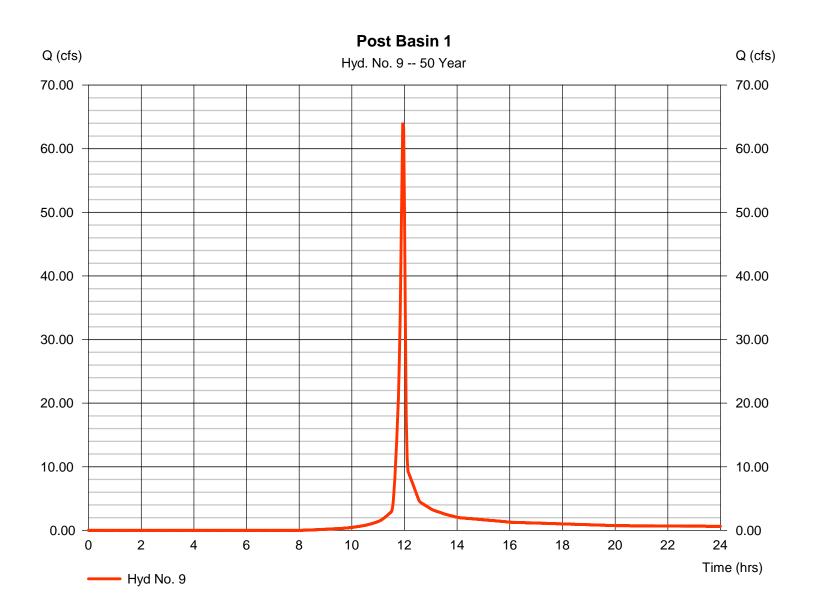
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Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 9

Post Basin 1

Hydrograph type	= SCS Runoff	Peak discharge	= 63.94 cfs
Storm frequency	= 50 yrs	Time to peak	= 11.93 hrs
Time interval	= 2 min	Hyd. volume	= 129,319 cuft
Drainage area	= 10.950 ac	Curve number	= 71.8
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 6.60 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



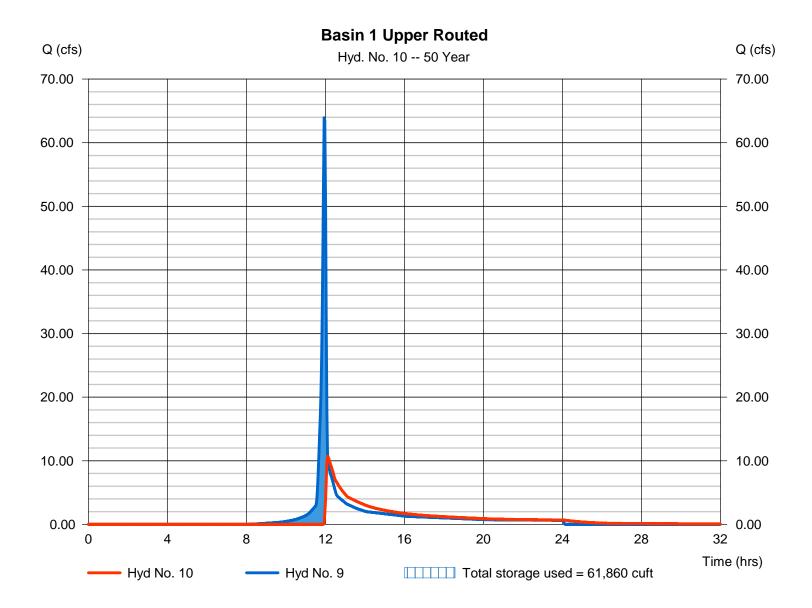
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 10

Basin 1 Upper Routed

Hydrograph type	= Reservoir	Peak discharge	= 10.69 cfs
Storm frequency	= 50 yrs	Time to peak	= 12.10 hrs
Time interval	= 2 min	Hyd. volume	= 91,830 cuft
Inflow hyd. No.	= 9 - Post Basin 1	Max. Elevation	= 323.71 ft
Reservoir name	= Basin 1 Upper	Max. Storage	= 61,860 cuft

Storage Indication method used.



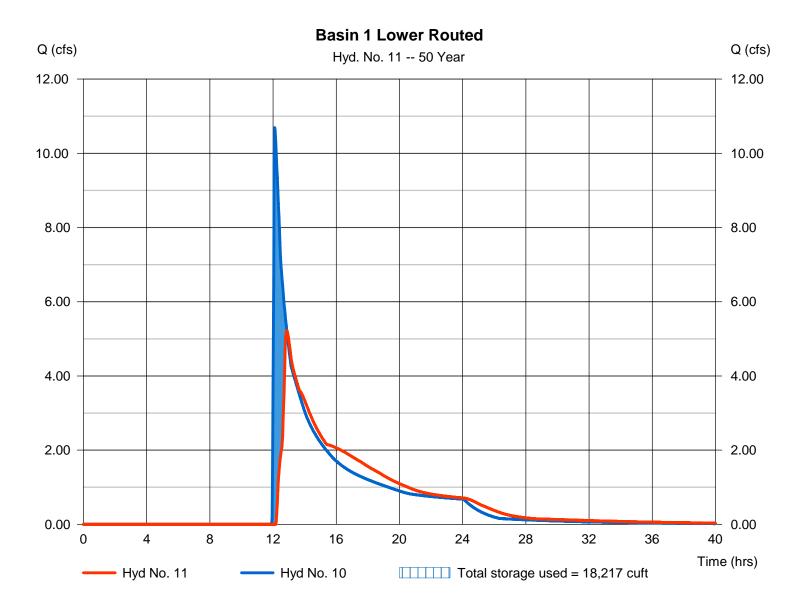
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 11

Basin 1 Lower Routed

Peak discharge	= 5.226 cfs
Time to peak	= 12.87 hrs
Hyd. volume	= 85,633 cuft
Jpper Routed Max. Elevation	= 305.66 ft
r Max. Storage	= 18,217 cuft
	Time to peak Hyd. volume Ipper Routed Max. Elevation

Storage Indication method used.

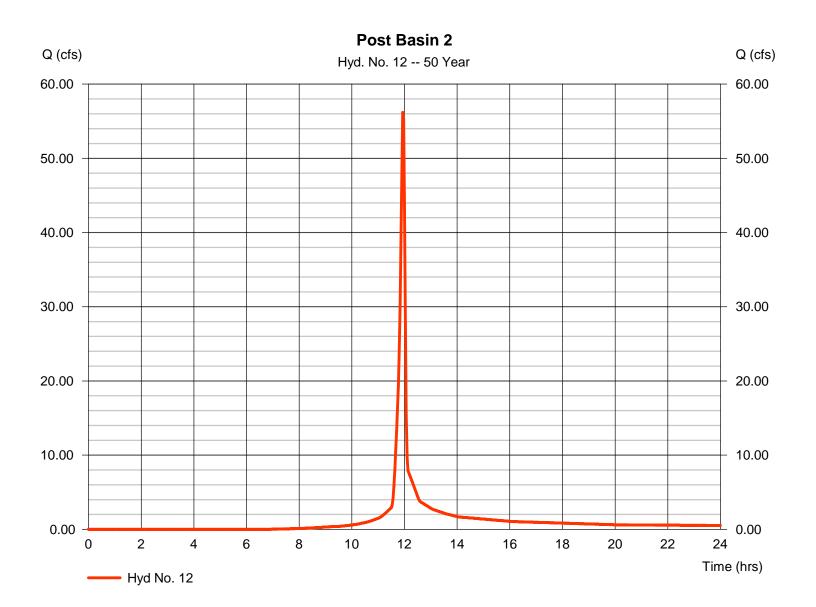


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 12

Post Basin 2

Hydrograph type	= SCS Runoff	Peak discharge	= 56.20 cfs
Storm frequency	= 50 yrs	Time to peak	= 11.93 hrs
Time interval	= 2 min	Hyd. volume	= 114,655 cuft
Drainage area	= 8.540 ac	Curve number	= 76.4
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 6.60 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484
		-	



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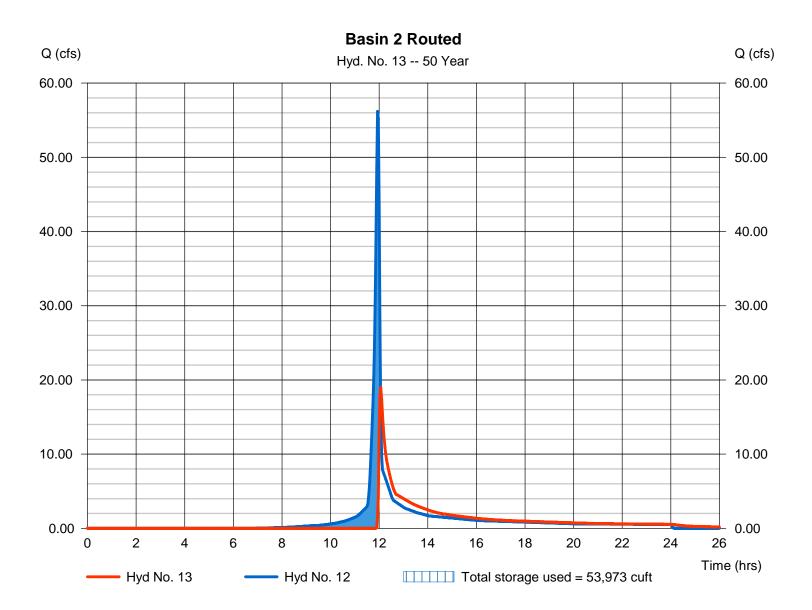
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 13

Basin 2 Routed

Hydrograph type	= Reservoir	Peak discharge	= 18.98 cfs
Storm frequency	= 50 yrs	Time to peak	= 12.07 hrs
Time interval	= 2 min	Hyd. volume	= 82,300 cuft
Inflow hyd. No.	= 12 - Post Basin 2	Max. Elevation	= 310.22 ft
Reservoir name	= Basin 2	Max. Storage	= 53,973 cuft

Storage Indication method used.

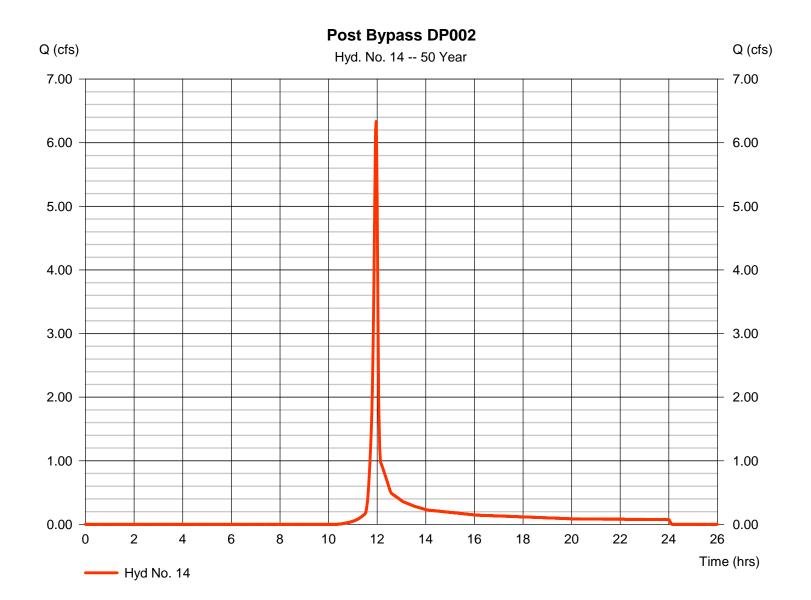


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 14

Post Bypass DP002

Hydrograph type	= SCS Runoff	Peak discharge	= 6.335 cfs
Storm frequency	= 50 yrs	Time to peak	= 11.97 hrs
Time interval	= 2 min	Hyd. volume	= 12,668 cuft
Drainage area	= 1.540 ac	Curve number	= 61
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 6.60 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

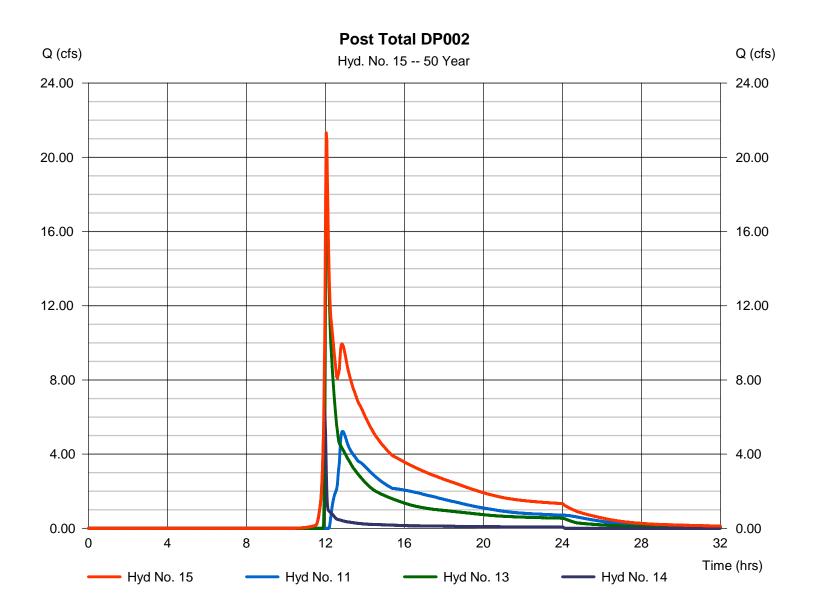


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 15

Post Total DP002

Combine 50 yrs 2 min 1, 13, 14	Time to peak = Hyd. volume =	= 21.32 cfs = 12.03 hrs = 180,602 cuft = 1.540 ac
1, 10, 14	Sonthis: drain: area -	- 1.040 ac
2	0 yrs min	0 yrs Time to peak = min Hyd. volume =



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Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

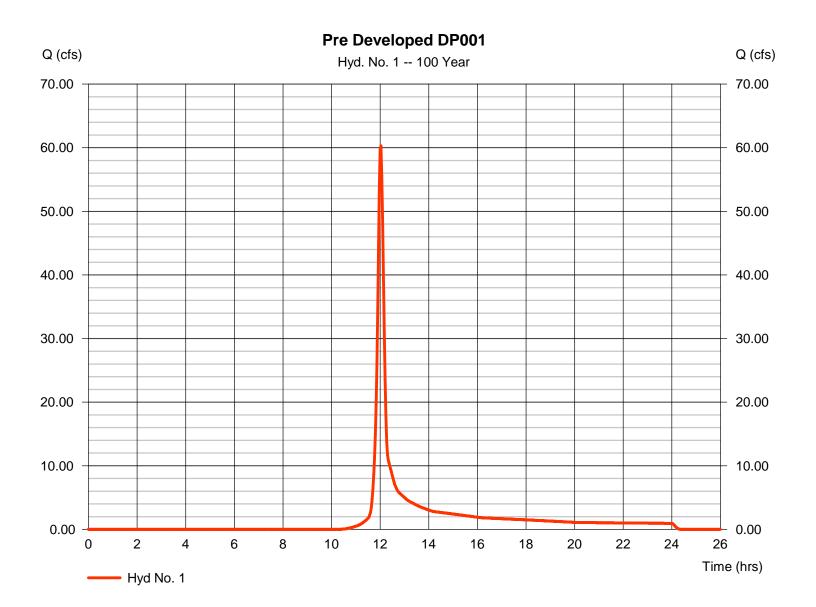
Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	60.31	2	722	159,136				Pre Developed DP001
2	SCS Runoff	79.34	2	722	208,579				Pre Developed DP002
4	SCS Runoff	84.16	2	716	170,598				Post Basin 3
5	Reservoir	24.38	2	724	125,131	4	319.74	74,844	Basin 3 Routed
6	SCS Runoff	8.052	2	718	16,178				Post Bypass DP001
7	Combine	29.04	2	718	141,309	5, 6			Post Total DP001
9	SCS Runoff	77.75	2	716	157,867				Post Basin 1
10	Reservoir	30.18	2	722	120,378	9	324.12	70,140	Basin 1 Upper Routed
11	Reservoir	13.13	2	738	114,181	10	305.96	20,385	Basin 1 Lower Routed
12	SCS Runoff	67.22	2	716	138,052				Post Basin 2
13	Reservoir	33.12	2	722	105,698	12	310.57	60,504	Basin 2 Routed
14	SCS Runoff	8.028	2	718	16,080				Post Bypass DP002
15	Combine	37.69	2	720	235,959	11, 13, 14			Post Total DP002
SW	/M.gpw				Return F	Period: 100	Year	Thursday,	09 / 30 / 2021

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 1

Pre Developed DP001

Hydrograph type	= SCS Runoff	Peak discharge	= 60.31 cfs
Storm frequency	= 100 yrs	Time to peak	= 12.03 hrs
Time interval	= 2 min	Hyd. volume	= 159,136 cuft
Drainage area	= 15.430 ac	Curve number	= 58
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= TR55	Time of conc. (Tc)	= 12.00 min
Total precip.	= 7.50 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

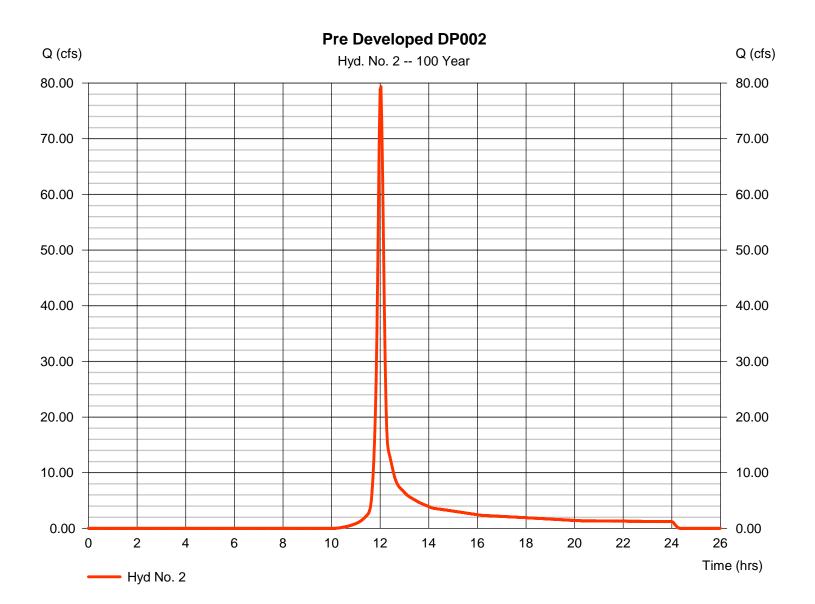


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 2

Pre Developed DP002

Hydrograph type	= SCS Runoff	Peak discharge	= 79.34 cfs
Storm frequency	= 100 yrs	Time to peak	= 12.03 hrs
Time interval	= 2 min	Hyd. volume	= 208,579 cuft
Drainage area	= 19.210 ac	Curve number	= 59.4
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= TR55	Time of conc. (Tc)	= 12.00 min
Total precip.	= 7.50 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

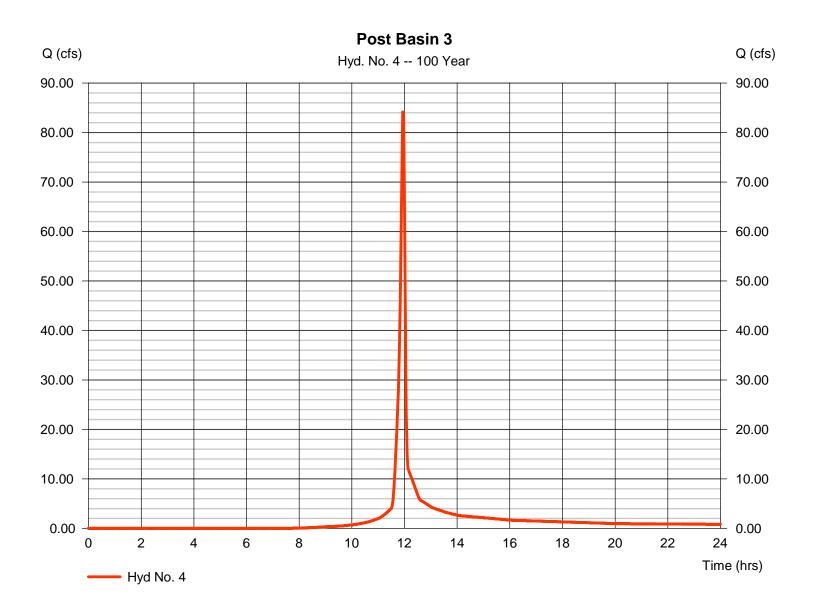


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 4

Post Basin 3

Hydrograph type	= SCS Runoff	Peak discharge	= 84.16 cfs
Storm frequency	= 100 yrs	Time to peak	= 11.93 hrs
Time interval	= 2 min	Hyd. volume	= 170,598 cuft
Drainage area	= 12.150 ac	Curve number	= 70.8
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 7.50 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



95

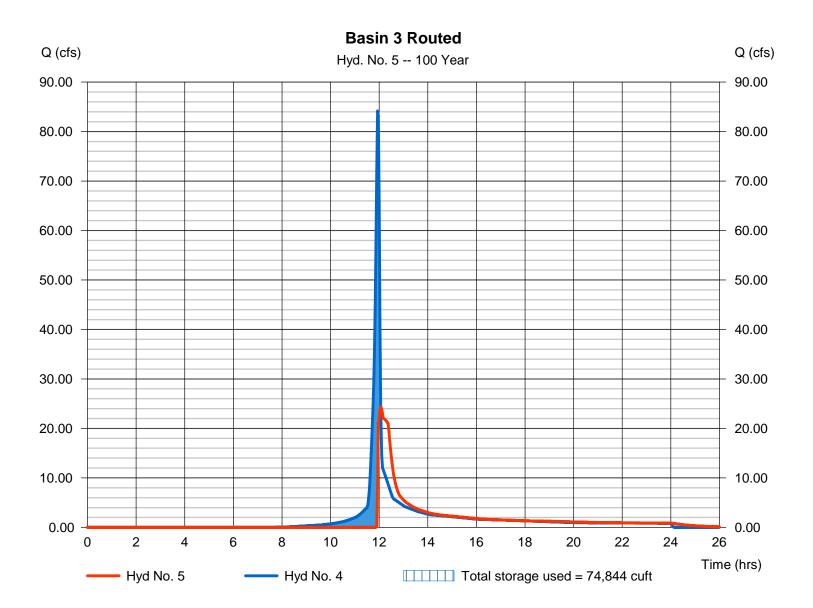
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 5

Basin 3 Routed

Hydrograph type	= Reservoir	Peak discharge	= 24.38 cfs
Storm frequency	= 100 yrs	Time to peak	= 12.07 hrs
Time interval	= 2 min	Hyd. volume	= 125,131 cuft
Inflow hyd. No.	= 4 - Post Basin 3	Max. Elevation	= 319.74 ft
Reservoir name	= Basin 3	Max. Storage	= 74,844 cuft

Storage Indication method used.

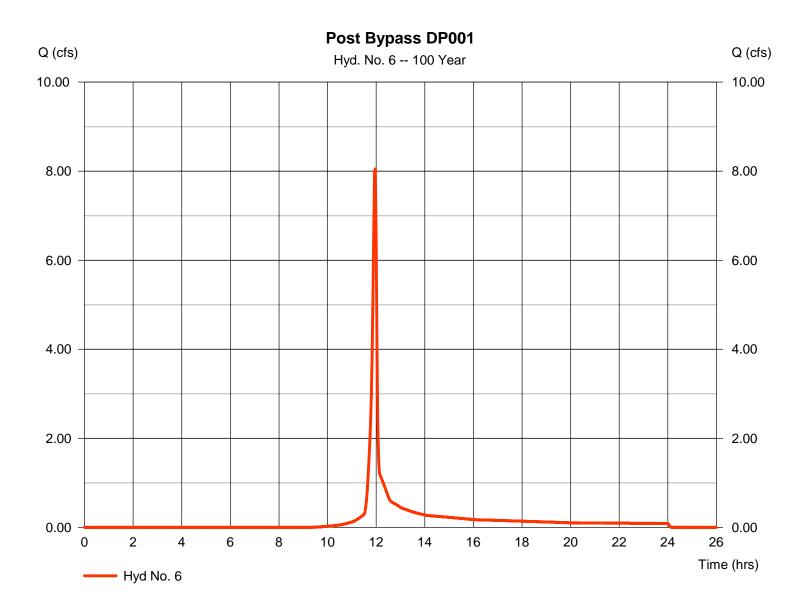


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 6

Post Bypass DP001

Hydrograph type	= SCS Runoff	Peak discharge	= 8.052 cfs
Storm frequency	= 100 yrs	Time to peak	= 11.97 hrs
Time interval	= 2 min	Hyd. volume	= 16,178 cuft
Drainage area	= 1.440 ac	Curve number	= 63.2
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 7.50 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

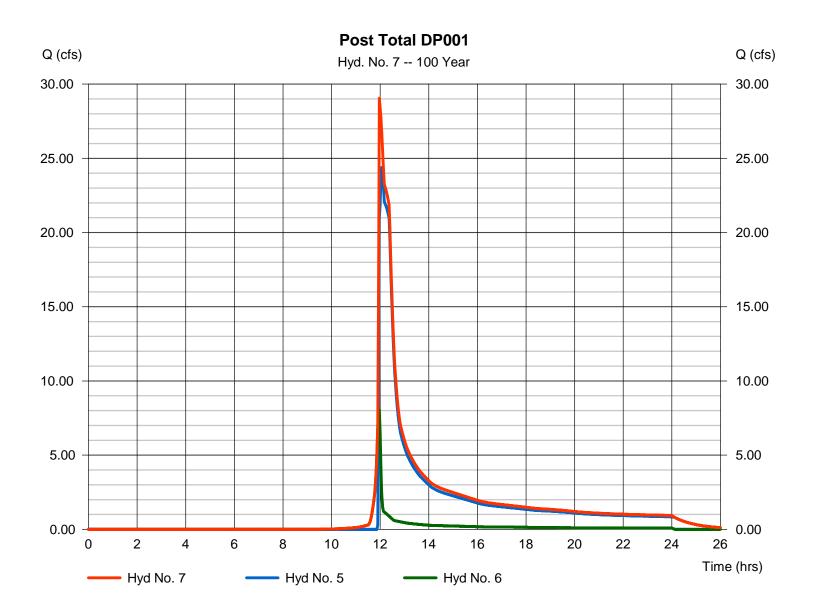


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 7

Post Total DP001

Inflow hyds. = 5, 6 Contrib. drain. area = 1.440 ac	Hydrograph type	= Combine	Peak discharge	= 29.04 cfs
	Storm frequency	= 100 yrs	Time to peak	= 11.97 hrs
	Time interval	= 2 min	Hyd. volume	= 141,309 cuft
	Inflow hyds.	= 5, 6	Contrib. drain. area	= 1.440 ac

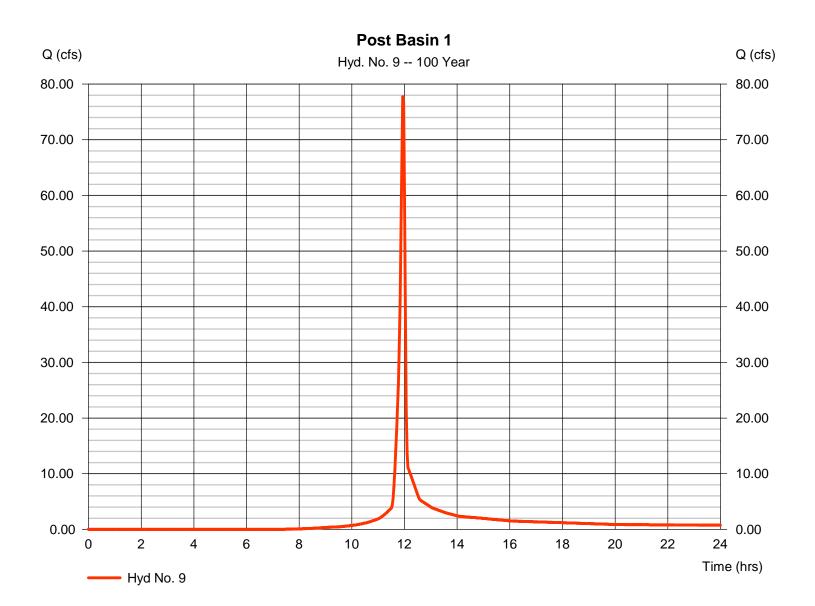


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 9

Post Basin 1

Hydrograph type	= SCS Runoff	Peak discharge	= 77.75 cfs
Storm frequency	= 100 yrs	Time to peak	= 11.93 hrs
Time interval	= 2 min	Hyd. volume	= 157,867 cuft
Drainage area	= 10.950 ac	Curve number	= 71.8
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 7.50 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



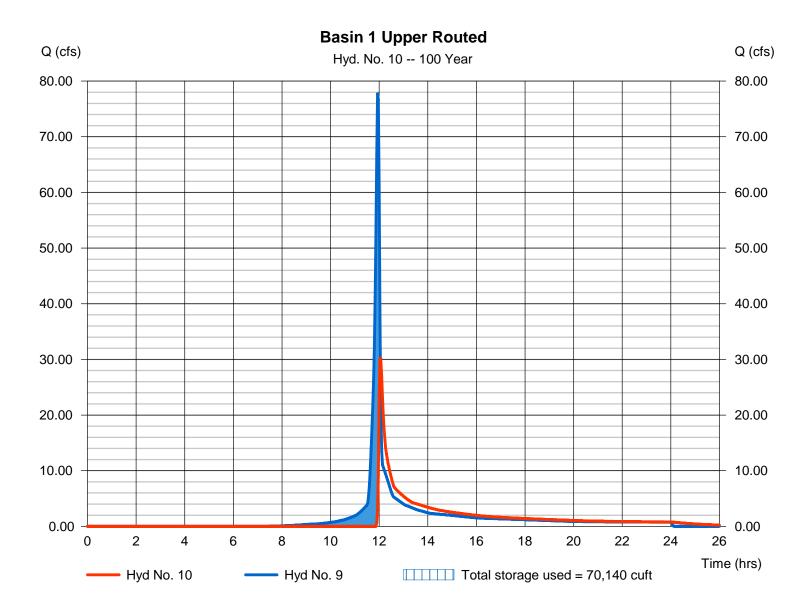
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 10

Basin 1 Upper Routed

Hydrograph type	= Reservoir	Peak discharge	= 30.18 cfs
Storm frequency	= 100 yrs	Time to peak	= 12.03 hrs
Time interval	= 2 min	Hyd. volume	= 120,378 cuft
Inflow hyd. No.	= 9 - Post Basin 1	Max. Elevation	= 324.12 ft
Reservoir name	= Basin 1 Upper	Max. Storage	= 70,140 cuft

Storage Indication method used.



100

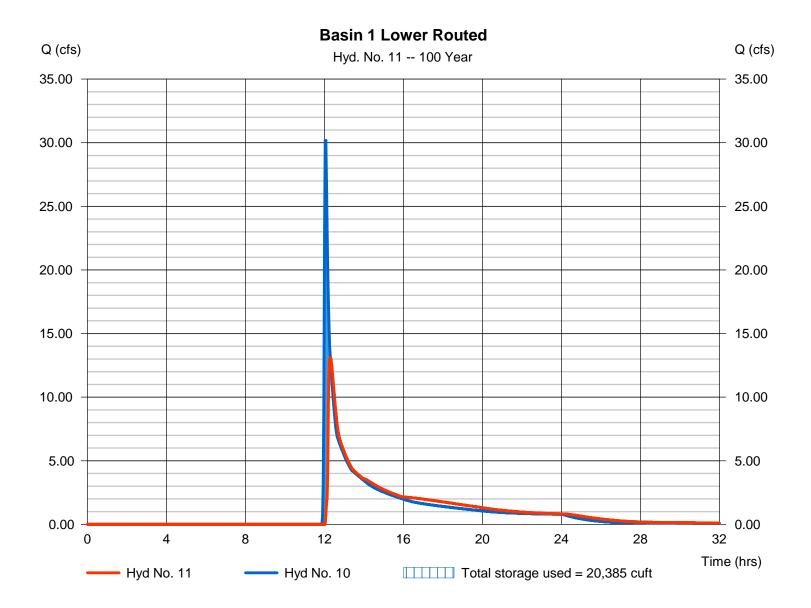
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 11

Basin 1 Lower Routed

Hydrograph type	= Reservoir	Peak discharge	= 13.13 cfs
Storm frequency	= 100 yrs	Time to peak	= 12.30 hrs
Time interval	= 2 min	Hyd. volume	= 114,181 cuft
Inflow hyd. No.	= 10 - Basin 1 Upper Routed	Max. Elevation	= 305.96 ft
Reservoir name	= Basin 1 Lower	Max. Storage	= 20,385 cuft

Storage Indication method used.

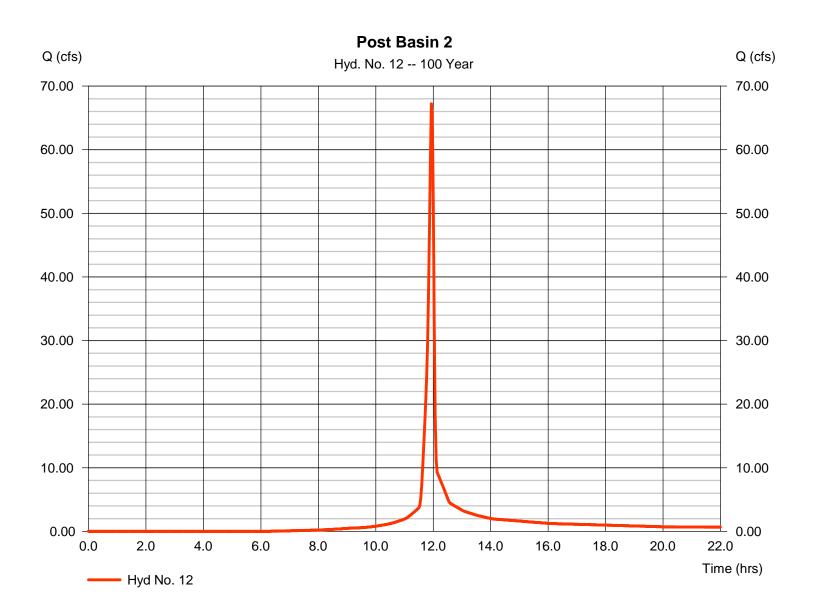


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 12

Post Basin 2

Hydrograph type	= SCS Runoff	Peak discharge	= 67.22 cfs
Storm frequency	= 100 yrs	Time to peak	= 11.93 hrs
Time interval	= 2 min	Hyd. volume	= 138,052 cuft
Drainage area	= 8.540 ac	Curve number	= 76.4
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 7.50 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



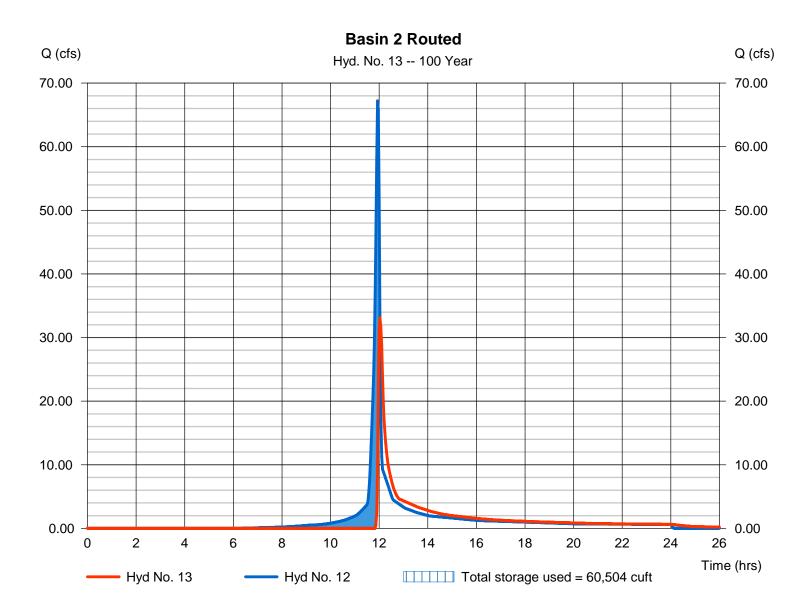
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 13

Basin 2 Routed

Hydrograph type	= Reservoir	Peak discharge	= 33.12 cfs
Storm frequency	= 100 yrs	Time to peak	= 12.03 hrs
Time interval	= 2 min	Hyd. volume	= 105,698 cuft
Inflow hyd. No.	= 12 - Post Basin 2	Max. Elevation	= 310.57 ft
Reservoir name	= Basin 2	Max. Storage	= 60,504 cuft

Storage Indication method used.

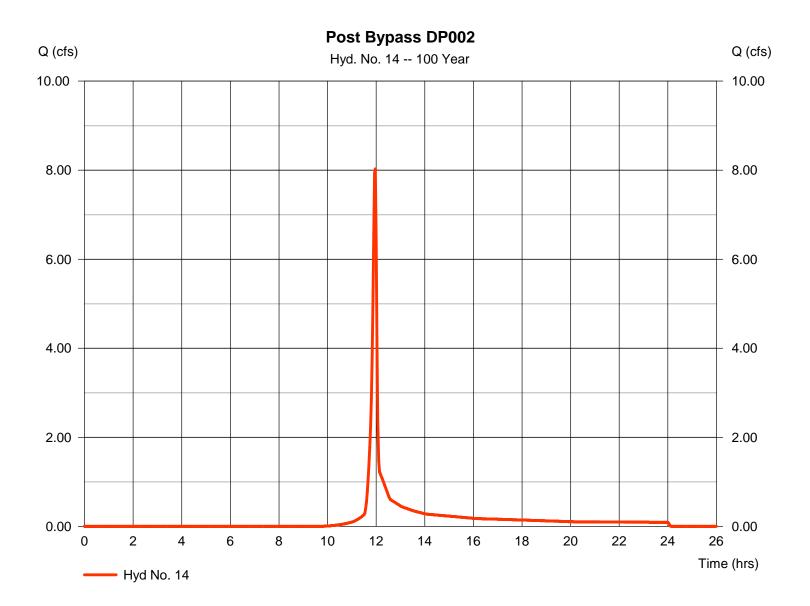


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 14

Post Bypass DP002

Hydrograph type	= SCS Runoff	Peak discharge	= 8.028 cfs
Storm frequency	= 100 yrs	Time to peak	= 11.97 hrs
Time interval	= 2 min	Hyd. volume	= 16,080 cuft
Drainage area	= 1.540 ac	Curve number	= 61
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 5.00 min
Total precip.	= 7.50 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

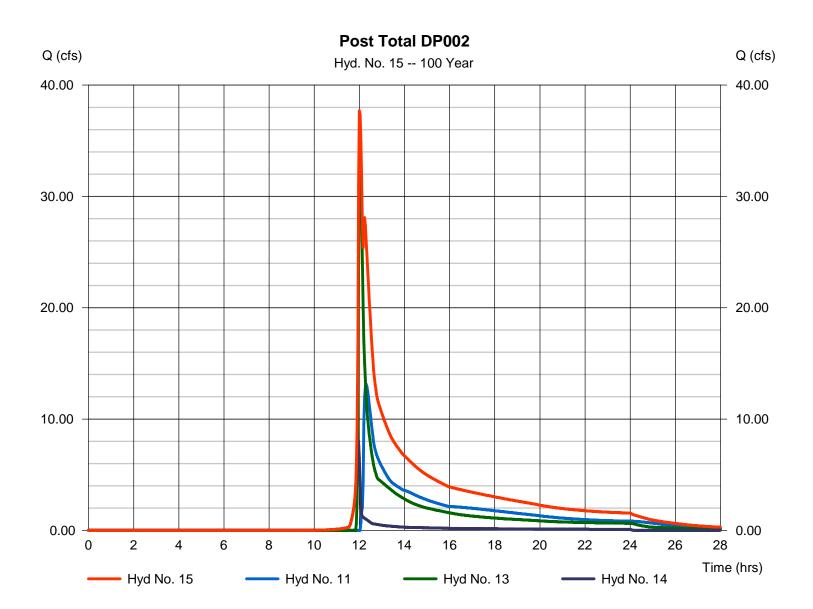


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Hyd. No. 15

Post Total DP002

Hydrograph type	= Combine	Peak discharge	 = 37.69 cfs = 12.00 hrs = 235,959 cuft = 1.540 ac
Storm frequency	= 100 yrs	Time to peak	
Time interval	= 2 min	Hyd. volume	
Inflow hyds.	= 11, 13, 14	Contrib. drain. area	
•			



APPENDIX E

USDA NRCS SOIL REPORT



United States Department of Agriculture

Natural Resources Conservation

Service

A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants

Custom Soil Resource Report for Chester County, Pennsylvania

Stokes



Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (https://offices.sc.egov.usda.gov/locator/app?agency=nrcs) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/? cid=nrcs142p2_053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

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How Soil Surveys Are Made

Soil surveys are made to provide information about the soils and miscellaneous areas in a specific area. They include a description of the soils and miscellaneous areas and their location on the landscape and tables that show soil properties and limitations affecting various uses. Soil scientists observed the steepness, length, and shape of the slopes; the general pattern of drainage; the kinds of crops and native plants; and the kinds of bedrock. They observed and described many soil profiles. A soil profile is the sequence of natural layers, or horizons, in a soil. The profile extends from the surface down into the unconsolidated material in which the soil formed or from the surface down to bedrock. The unconsolidated material is devoid of roots and other living organisms and has not been changed by other biological activity.

Currently, soils are mapped according to the boundaries of major land resource areas (MLRAs). MLRAs are geographically associated land resource units that share common characteristics related to physiography, geology, climate, water resources, soils, biological resources, and land uses (USDA, 2006). Soil survey areas typically consist of parts of one or more MLRA.

The soils and miscellaneous areas in a survey area occur in an orderly pattern that is related to the geology, landforms, relief, climate, and natural vegetation of the area. Each kind of soil and miscellaneous area is associated with a particular kind of landform or with a segment of the landform. By observing the soils and miscellaneous areas in the survey area and relating their position to specific segments of the landform, a soil scientist develops a concept, or model, of how they were formed. Thus, during mapping, this model enables the soil scientist to predict with a considerable degree of accuracy the kind of soil or miscellaneous area at a specific location on the landscape.

Commonly, individual soils on the landscape merge into one another as their characteristics gradually change. To construct an accurate soil map, however, soil scientists must determine the boundaries between the soils. They can observe only a limited number of soil profiles. Nevertheless, these observations, supplemented by an understanding of the soil-vegetation-landscape relationship, are sufficient to verify predictions of the kinds of soil in an area and to determine the boundaries.

Soil scientists recorded the characteristics of the soil profiles that they studied. They noted soil color, texture, size and shape of soil aggregates, kind and amount of rock fragments, distribution of plant roots, reaction, and other features that enable them to identify soils. After describing the soils in the survey area and determining their properties, the soil scientists assigned the soils to taxonomic classes (units). Taxonomic classes are concepts. Each taxonomic class has a set of soil characteristics with precisely defined limits. The classes are used as a basis for comparison to classify soils systematically. Soil taxonomy, the system of taxonomic classification used in the United States, is based mainly on the kind and character of soil properties and the arrangement of horizons within the profile. After the soil

scientists classified and named the soils in the survey area, they compared the individual soils with similar soils in the same taxonomic class in other areas so that they could confirm data and assemble additional data based on experience and research.

The objective of soil mapping is not to delineate pure map unit components; the objective is to separate the landscape into landforms or landform segments that have similar use and management requirements. Each map unit is defined by a unique combination of soil components and/or miscellaneous areas in predictable proportions. Some components may be highly contrasting to the other components of the map unit. The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The delineation of such landforms and landform segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, onsite investigation is needed to define and locate the soils and miscellaneous areas.

Soil scientists make many field observations in the process of producing a soil map. The frequency of observation is dependent upon several factors, including scale of mapping, intensity of mapping, design of map units, complexity of the landscape, and experience of the soil scientist. Observations are made to test and refine the soil-landscape model and predictions and to verify the classification of the soils at specific locations. Once the soil-landscape model is refined, a significantly smaller number of measurements of individual soil properties are made and recorded. These measurements may include field measurements, such as those for color, depth to bedrock, and texture, and laboratory measurements, such as those for content of sand, silt, clay, salt, and other components. Properties of each soil typically vary from one point to another across the landscape.

Observations for map unit components are aggregated to develop ranges of characteristics for the components. The aggregated values are presented. Direct measurements do not exist for every property presented for every map unit component. Values for some properties are estimated from combinations of other properties.

While a soil survey is in progress, samples of some of the soils in the area generally are collected for laboratory analyses and for engineering tests. Soil scientists interpret the data from these analyses and tests as well as the field-observed characteristics and the soil properties to determine the expected behavior of the soils under different uses. Interpretations for all of the soils are field tested through observation of the soils in different uses and under different levels of management. Some interpretations are modified to fit local conditions, and some new interpretations are developed to meet local needs. Data are assembled from other sources, such as research information, production records, and field experience of specialists. For example, data on crop yields under defined levels of management are assembled from farm records and from field or plot experiments on the same kinds of soil.

Predictions about soil behavior are based not only on soil properties but also on such variables as climate and biological activity. Soil conditions are predictable over long periods of time, but they are not predictable from year to year. For example, soil scientists can predict with a fairly high degree of accuracy that a given soil will have a high water table within certain depths in most years, but they cannot predict that a high water table will always be at a specific level in the soil on a specific date.

After soil scientists located and identified the significant natural bodies of soil in the survey area, they drew the boundaries of these bodies on aerial photographs and

identified each as a specific map unit. Aerial photographs show trees, buildings, fields, roads, and rivers, all of which help in locating boundaries accurately.

Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.



	Amage of Interest (ACI) Image of In			MAP INFORMATION
Area of Int	. ,			The soil surveys that comprise your AOI were mapped at 1:24,000.
Soils	Soil Map Unit Polygons			Warning: Soil Map may not be valid at this scale.
	·	Δ	Other	Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of
ల	Blowout		tures	contrasting soils that could have been shown at a more detailed scale.
×	Clay Spot	•		Please rely on the bar scale on each map sheet for map measurements.
X	Gravel Pit	~	U	Source of Map: Natural Resources Conservation Service Web Soil Survey URL: Coordinate System: Web Mercator (EPSG:3857)
0	Landfill	~	Local Roads	Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts
<u>ىل</u> ە	Marsh or swamp	Backgrour		distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.
0				This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.
~				Soil Survey Area: Chester County, Pennsylvania Survey Area Data: Version 13, Jun 5, 2020
0 0 0 0	5 .			Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.
\$ \$	Sinkhole Slide or Slip			Date(s) aerial images were photographed: May 26, 2019—Jul 10, 2019
ß	Sodic Spot			The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
Ва	Baile silt loam	11.2	16.9%
Со	Codorus silt loam	1.1	1.6%
GdB	Gladstone gravelly loam, 3 to 8 percent slopes	6.0	9.1%
GdC	Gladstone gravelly loam, 8 to 15 percent slopes	32.3	49.0%
GfD	Gladstone gravelly loam, 8 to 25 percent slopes, very bouldery	5.5	8.4%
На	Hatboro silt loam	6.0	9.1%
MaD Manor loam, 15 to 25 percent slopes		3.8	5.8%
UrlB	Urban land-Gladstone complex, 0 to 8 percent slopes	0.1	0.1%
Totals for Area of Interest		66.0	100.0%

Map Unit Legend

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit

descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

Chester County, Pennsylvania

Ba—Baile silt loam

Map Unit Setting

National map unit symbol: pjb7 Elevation: 200 to 2,000 feet Mean annual precipitation: 35 to 55 inches Mean annual air temperature: 45 to 61 degrees F Frost-free period: 110 to 235 days Farmland classification: Not prime farmland

Map Unit Composition

Baile and similar soils: 85 percent Minor components: 15 percent Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Baile

Setting

Landform: Depressions Landform position (two-dimensional): Footslope Landform position (three-dimensional): Base slope Down-slope shape: Linear, concave Across-slope shape: Concave, linear Parent material: Local alluvium over residuum weathered from mica schist

Typical profile

Ap - 0 to 10 inches: silt loam Btg - 10 to 40 inches: silt loam Cg - 40 to 60 inches: loam

Properties and qualities

Slope: 0 to 3 percent
Depth to restrictive feature: 60 to 99 inches to lithic bedrock
Drainage class: Poorly drained
Runoff class: Very high
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to moderately high (0.06 to 0.20 in/hr)
Depth to water table: About 0 to 6 inches
Frequency of flooding: None
Frequency of ponding: None
Available water capacity: High (about 11.0 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 5w Hydrologic Soil Group: C/D Hydric soil rating: Yes

Minor Components

Glenville

Percent of map unit: 9 percent Landform: Hillslopes Landform position (two-dimensional): Footslope, backslope Landform position (three-dimensional): Side slope, head slope Down-slope shape: Linear, concave Across-slope shape: Concave, linear Hydric soil rating: No

Manor

Percent of map unit: 2 percent Landform: Hills Landform position (two-dimensional): Shoulder, backslope Landform position (three-dimensional): Side slope, nose slope Down-slope shape: Linear, convex Across-slope shape: Convex, linear Hydric soil rating: No

Chester

Percent of map unit: 2 percent Landform: Hills Landform position (two-dimensional): Summit, shoulder Landform position (three-dimensional): Interfluve Down-slope shape: Linear, convex Across-slope shape: Convex, linear Hydric soil rating: No

Glenelg

Percent of map unit: 2 percent Landform: Hillslopes Landform position (two-dimensional): Summit, shoulder, backslope Landform position (three-dimensional): Interfluve, side slope Down-slope shape: Linear, convex Across-slope shape: Convex, linear Hydric soil rating: No

Co—Codorus silt loam

Map Unit Setting

National map unit symbol: pjfx Elevation: 200 to 2,000 feet Mean annual precipitation: 35 to 50 inches Mean annual air temperature: 45 to 57 degrees F Frost-free period: 120 to 220 days Farmland classification: All areas are prime farmland

Map Unit Composition

Codorus and similar soils: 85 percent Minor components: 15 percent Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Codorus

Setting

Landform: Flood plains

Landform position (two-dimensional): Toeslope

Landform position (three-dimensional): Tread

Down-slope shape: Linear

Across-slope shape: Linear

Parent material: Alluvium derived from gneiss and/or alluvium derived from mica schist

Typical profile

Ap - 0 to 12 inches: silt loam *Bw - 12 to 48 inches:* silt loam *C - 48 to 60 inches:* silt loam

Properties and qualities

Slope: 0 to 3 percent
Depth to restrictive feature: 72 to 99 inches to lithic bedrock
Drainage class: Moderately well drained
Runoff class: Low
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.60 to 2.00 in/hr)
Depth to water table: About 18 to 36 inches
Frequency of flooding: OccasionalNone
Frequency of ponding: None
Available water capacity: Moderate (about 8.5 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 2w Hydrologic Soil Group: C Hydric soil rating: No

Minor Components

Hatboro

Percent of map unit: 8 percent Landform: Flood plains Landform position (two-dimensional): Toeslope Landform position (three-dimensional): Tread Down-slope shape: Concave, linear Across-slope shape: Concave, linear Hydric soil rating: Yes

Glenville

Percent of map unit: 4 percent Landform: Hillslopes Landform position (two-dimensional): Footslope, backslope Landform position (three-dimensional): Side slope, head slope Down-slope shape: Linear, concave Across-slope shape: Concave, linear Hydric soil rating: No

Baile

Percent of map unit: 3 percent Landform: Depressions Landform position (two-dimensional): Footslope Landform position (three-dimensional): Base slope Down-slope shape: Linear, concave Across-slope shape: Concave, linear Hydric soil rating: Yes

GdB—Gladstone gravelly loam, 3 to 8 percent slopes

Map Unit Setting

National map unit symbol: 2v7gk Elevation: 250 to 1,200 feet Mean annual precipitation: 30 to 64 inches Mean annual air temperature: 46 to 79 degrees F Frost-free period: 131 to 178 days Farmland classification: All areas are prime farmland

Map Unit Composition

Gladstone and similar soils: 85 percent *Minor components:* 15 percent *Estimates are based on observations, descriptions, and transects of the mapunit.*

Description of Gladstone

Setting

Landform: Hills Landform position (two-dimensional): Shoulder Landform position (three-dimensional): Side slope Down-slope shape: Linear Across-slope shape: Convex Parent material: Loamy colluvium derived from granite and gneiss and/or loamy residuum weathered from granite and gneiss

Typical profile

Ap - 0 to 10 inches: gravelly loam Bt1 - 10 to 22 inches: sandy clay loam Bt2 - 22 to 37 inches: loam C - 37 to 66 inches: sandy loam R - 66 to 76 inches: bedrock

Properties and qualities

Slope: 3 to 8 percent
Depth to restrictive feature: 60 to 80 inches to lithic bedrock
Drainage class: Well drained
Capacity of the most limiting layer to transmit water (Ksat): Very low to moderately low (0.00 to 0.06 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water capacity: Moderate (about 8.4 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 2e Hydrologic Soil Group: B Hydric soil rating: No

Minor Components

Califon

Percent of map unit: 5 percent Landform: Flats Landform position (two-dimensional): Footslope Landform position (three-dimensional): Base slope Down-slope shape: Concave Across-slope shape: Linear Hydric soil rating: No

Annandale

Percent of map unit: 5 percent Landform: Hills Landform position (two-dimensional): Summit Landform position (three-dimensional): Interfluve Down-slope shape: Convex Across-slope shape: Linear Hydric soil rating: No

Parker

Percent of map unit: 5 percent Landform: Hills Landform position (two-dimensional): Shoulder Landform position (three-dimensional): Side slope Down-slope shape: Convex Across-slope shape: Linear Hydric soil rating: No

GdC—Gladstone gravelly loam, 8 to 15 percent slopes

Map Unit Setting

National map unit symbol: 2v7gl Elevation: 250 to 1,200 feet Mean annual precipitation: 30 to 64 inches Mean annual air temperature: 46 to 79 degrees F Frost-free period: 170 to 240 days Farmland classification: Farmland of statewide importance

Map Unit Composition

Gladstone and similar soils: 85 percent *Minor components:* 15 percent *Estimates are based on observations, descriptions, and transects of the mapunit.*

Description of Gladstone

Setting

Landform: Hillslopes Landform position (two-dimensional): Shoulder Landform position (three-dimensional): Side slope Down-slope shape: Linear

Across-slope shape: Convex

Parent material: Loamy colluvium derived from granite and gneiss and/or loamy residuum weathered from granite and gneiss

Typical profile

Ap - 0 to 10 inches: gravelly loam Bt1 - 10 to 22 inches: gravelly sandy clay loam Bt2 - 22 to 37 inches: gravelly loam C - 37 to 66 inches: gravelly sandy loam R - 66 to 76 inches: bedrock

Properties and qualities

Slope: 8 to 15 percent
Depth to restrictive feature: 65 to 67 inches to lithic bedrock
Drainage class: Well drained
Capacity of the most limiting layer to transmit water (Ksat): Very low to high (0.00 to 5.95 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water capacity: Moderate (about 6.1 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 3e Hydrologic Soil Group: B Hydric soil rating: No

Minor Components

Parker

Percent of map unit: 5 percent Landform: Hillslopes Landform position (two-dimensional): Shoulder Landform position (three-dimensional): Side slope Down-slope shape: Convex Across-slope shape: Linear Hydric soil rating: No

Califon

Percent of map unit: 5 percent Landform: Flats Landform position (two-dimensional): Footslope Landform position (three-dimensional): Base slope Down-slope shape: Concave Across-slope shape: Linear Hydric soil rating: No

Annandale

Percent of map unit: 5 percent Landform: Hillslopes Landform position (two-dimensional): Summit Landform position (three-dimensional): Interfluve Down-slope shape: Convex Across-slope shape: Linear Hydric soil rating: No

GfD—Gladstone gravelly loam, 8 to 25 percent slopes, very bouldery

Map Unit Setting

National map unit symbol: wphh Elevation: 200 to 1,200 feet Mean annual precipitation: 40 to 48 inches Mean annual air temperature: 45 to 55 degrees F Frost-free period: 150 to 190 days Farmland classification: Not prime farmland

Map Unit Composition

Gladstone, very bouldery, and similar soils: 90 percent *Minor components:* 10 percent *Estimates are based on observations, descriptions, and transects of the mapunit.*

Description of Gladstone, Very Bouldery

Setting

Landform: Hillslopes Landform position (two-dimensional): Summit, shoulder Landform position (three-dimensional): Nose slope, side slope Down-slope shape: Linear, convex Across-slope shape: Linear, convex Parent material: Local colluvium and residuum weathered from granite and gneiss

Typical profile

A - 0 to 10 inches: gravelly loam Bt - 10 to 42 inches: gravelly clay loam C - 42 to 68 inches: very gravelly loam R - 68 to 78 inches: bedrock

Properties and qualities

Slope: 8 to 25 percent
Surface area covered with cobbles, stones or boulders: 1.6 percent
Depth to restrictive feature: 60 to 100 inches to lithic bedrock
Drainage class: Well drained
Runoff class: Low
Capacity of the most limiting layer to transmit water (Ksat): Very low to high (0.00 to 6.00 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water capacity: Moderate (about 6.9 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 6s Hydrologic Soil Group: A Hydric soil rating: No

Minor Components

Cokesbury

Percent of map unit: 5 percent Landform: Depressions Landform position (two-dimensional): Toeslope, footslope Landform position (three-dimensional): Base slope Down-slope shape: Concave Across-slope shape: Concave Hydric soil rating: Yes

Califon

Percent of map unit: 5 percent Landform: Hills Landform position (two-dimensional): Backslope, footslope Landform position (three-dimensional): Head slope Down-slope shape: Concave, linear Across-slope shape: Concave, linear Hydric soil rating: No

Ha—Hatboro silt loam

Map Unit Setting

National map unit symbol: 1lwqq Elevation: 200 to 800 feet Mean annual precipitation: 36 to 50 inches Mean annual air temperature: 48 to 57 degrees F Frost-free period: 140 to 200 days Farmland classification: Not prime farmland

Map Unit Composition

Hatboro and similar soils: 95 percent Minor components: 5 percent Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Hatboro

Setting

Landform: Flood plains Landform position (two-dimensional): Toeslope Landform position (three-dimensional): Tread Down-slope shape: Concave, linear Across-slope shape: Concave, linear Parent material: Alluvium derived from metamorphic and sedimentary rock

Typical profile

Ap - 0 to 9 inches: silt loam Bg - 9 to 44 inches: silt loam Cg - 44 to 56 inches: sandy clay loam C - 56 to 70 inches: stratified gravelly sand to clay

Properties and qualities

Slope: 0 to 3 percent
Depth to restrictive feature: 60 to 99 inches to lithic bedrock
Drainage class: Poorly drained
Runoff class: Very high
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.60 to 2.00 in/hr)
Depth to water table: About 0 to 6 inches
Frequency of flooding: FrequentNone
Frequency of ponding: None
Available water capacity: High (about 9.7 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 4w Hydrologic Soil Group: B/D Hydric soil rating: Yes

Minor Components

Glenville

Percent of map unit: 5 percent Landform: Hillslopes Landform position (two-dimensional): Footslope, backslope Landform position (three-dimensional): Side slope, head slope Down-slope shape: Linear, concave Across-slope shape: Concave, linear Hydric soil rating: No

MaD—Manor loam, 15 to 25 percent slopes

Map Unit Setting

National map unit symbol: 2tmcg Elevation: 250 to 1,000 feet Mean annual precipitation: 40 to 55 inches Mean annual air temperature: 48 to 57 degrees F Frost-free period: 150 to 192 days Farmland classification: Not prime farmland

Map Unit Composition

Manor and similar soils: 85 percent Minor components: 15 percent Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Manor

Setting

Landform: Hillslopes Landform position (two-dimensional): Shoulder, backslope, summit Landform position (three-dimensional): Side slope *Down-slope shape:* Convex *Across-slope shape:* Convex *Parent material:* Residuum weathered from mica schist

Typical profile

A1 - 0 to 2 inches: loam A2 - 2 to 6 inches: sandy loam Bw1 - 6 to 13 inches: fine sandy loam Bw2 - 13 to 22 inches: fine sandy loam C1 - 22 to 30 inches: fine sandy loam C2 - 30 to 44 inches: channery coarse sand C3 - 44 to 53 inches: loamy sand C4 - 53 to 83 inches: channery loamy sand Cr - 83 to 108 inches: bedrock

R - 108 to 138 inches: bedrock

Properties and qualities

Slope: 15 to 25 percent
Surface area covered with cobbles, stones or boulders: 0.0 percent
Depth to restrictive feature: 59 to 100 inches to paralithic bedrock; 100 to 128 inches to lithic bedrock
Drainage class: Well drained
Capacity of the most limiting layer to transmit water (Ksat): Moderately low (0.01 to 0.07 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water capacity: Moderate (about 8.0 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 4e Hydrologic Soil Group: B Hydric soil rating: No

Minor Components

Glenville

Percent of map unit: 5 percent Landform: Drainageways, swales Landform position (two-dimensional): Footslope, backslope Landform position (three-dimensional): Base slope, head slope, interfluve Down-slope shape: Linear Across-slope shape: Concave Hydric soil rating: No

Mt. airy

Percent of map unit: 5 percent Landform: Hillslopes Landform position (two-dimensional): Backslope, summit, shoulder Landform position (three-dimensional): Nose slope Down-slope shape: Convex Across-slope shape: Convex Hydric soil rating: No

Blocktown

Percent of map unit: 5 percent

Custom Soil Resource Report

Landform: Hillslopes Landform position (two-dimensional): Backslope Landform position (three-dimensional): Side slope Down-slope shape: Convex Across-slope shape: Convex Hydric soil rating: No

UrlB—Urban land-Gladstone complex, 0 to 8 percent slopes

Map Unit Setting

National map unit symbol: 1r3cq Elevation: 200 to 1,200 feet Mean annual precipitation: 36 to 48 inches Mean annual air temperature: 44 to 57 degrees F Frost-free period: 130 to 190 days Farmland classification: Not prime farmland

Map Unit Composition

Urban land: 65 percent *Gladstone and similar soils:* 25 percent *Minor components:* 10 percent *Estimates are based on observations, descriptions, and transects of the mapunit.*

Description of Urban Land

Setting

Landform: Hills Down-slope shape: Linear Across-slope shape: Linear Parent material: Pavement, buildings and other artifically covered areas

Typical profile

C - 0 to 6 inches: variable

Properties and qualities

Slope: 0 to 8 percent *Depth to restrictive feature:* 10 to 100 inches to lithic bedrock *Available water capacity:* Very low (about 0.0 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 8s Hydric soil rating: No

Description of Gladstone

Setting

Landform: Hillslopes Landform position (two-dimensional): Summit, shoulder Landform position (three-dimensional): Nose slope, side slope Down-slope shape: Linear, convex Across-slope shape: Linear, convex Parent material: Local colluvium and residuum weathered from granite and gneiss

Typical profile

A - 0 to 10 inches: gravelly loam C - 10 to 42 inches: gravelly clay loam 2Ap - 42 to 68 inches: gravelly loam R - 68 to 78 inches: bedrock

Properties and qualities

Slope: 0 to 8 percent
Depth to restrictive feature: 60 to 100 inches to lithic bedrock
Drainage class: Well drained
Runoff class: Very low
Capacity of the most limiting layer to transmit water (Ksat): Very low to high (0.00 to 6.00 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water capacity: Moderate (about 6.9 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 2e Hydrologic Soil Group: A Hydric soil rating: No

Minor Components

Cokesbury

Percent of map unit: 5 percent Landform: Depressions Landform position (two-dimensional): Toeslope, footslope Landform position (three-dimensional): Base slope Down-slope shape: Concave Across-slope shape: Concave Hydric soil rating: Yes

Califon

Percent of map unit: 5 percent Landform: Hills Landform position (two-dimensional): Backslope, footslope Landform position (three-dimensional): Head slope Down-slope shape: Concave, linear Across-slope shape: Concave, linear Hydric soil rating: No

APPENDIX E

STORMWATER INFILTRATION TESTING REPORT

STORMWATER INFILTRATION REPORT

FOR

1013 SHILOH ROAD WESTTOWN TOWNSHIP CHESTER COUNTY

PREPARED FOR:

Keystone Custom Homes

PREPARED BY:

D.L. Howell & Associates, Inc. 1250 Wrights Lane West Chester, PA 19380

March 2021



Stormwater Infiltration Test Report 1013 Shiloh Road Westtown Township Chester County

On Monday and Tuesday, March 22-23, 2021, D.L. Howell and Associates, Inc. preformed hydraulic conductivity tests for the proposed stormwater management areas for the property located at 1013 Shiloh Road in Westtown Township, Chester County. The purpose of the hydraulic conductivity testing was to determine site suitability for the proposed stormwater infiltration areas associated with proposed improvements at the site (see development plan).

Testing was conducted in general accordance with the Pennsylvania Department of Environmental Protection (PADEP)'s Pennsylvania Stormwater Best Management Practices Manual specifications, in a cased, sealed, borehole utilizing the falling head method designed to measure the vertical hydraulic conductivity of the soil. An approximate five-inch diameter borehole was hand dug to the depth of the proposed bottom elevation of the infiltration structure and a 3-inch diameter PVC casing was installed. A mixture of bentonite and soil was placed around the annulus of the casing and packed to seal the casing in place. The casing was presoaked immediately prior to the start of the test to simulate field saturated conditions. A measured amount of water was poured into the sealed casing to begin the 30-minute presoak. After the final 30-minute presoaking period, the water in the casing was adjusted to a known depth and consecutively re-adjusted after each reading and the drop of the water column is measured. The test continued until the readings became stabilized or for a maximum of eight readings. A stabilized rate of drop means a difference of ¹/₄ inch or less of drop between the highest and lowest readings of four consecutive readings.

Within the site, four hydraulic conductivity tests were conducted at the elevations associated with the proposed bottom of the infiltration structures. One deep test pit was excavated at each infiltration test location to identify limiting conditions such as mottling, depth of bedrock, and depth of groundwater. Testing was to be conducted within the footprint of the proposed infiltration structures.

• Infiltration Test 3-23-1 was conducted at approximately \pm 5.0 feet below existing grade, which corresponds to an approximate infiltration elevation of 317.0. One deep test pit was excavated at this location to a depth of 7.0 feet below existing grade. During excavation, rock was encountered at a depth of approximately 7.0 feet below existing grade.

Based on the hydraulic conductivity testing located within the footprint of the infiltration structure, D.L. Howell & Associates, Inc., recommends the following infiltration rate for the soils underlying Test 3-23-1: an infiltration rate of 1.50 inches per hour shall be used.

• Infiltration Test 3-23-2 was conducted at approximately \pm 4.0 feet below existing grade, which corresponds to an approximate infiltration elevation of 314.0. One deep test pit was excavated at this location to a depth of 6.0 feet below existing grade. No limiting conditions were identified at the time of excavation.

Based on the hydraulic conductivity testing located within the footprint of the infiltration structure, D.L. Howell & Associates, Inc., recommends the following infiltration rate for the soils underlying Test 3-23-2: an infiltration rate of 1.14 inches per hour shall be used.

• Infiltration Test 3-23-3 was conducted at approximately \pm 3.5 feet below existing grade, which corresponds to an approximate infiltration elevation of 303.5. One deep test pit was excavated at this location to a depth of 5.5 feet below existing grade. During excavation, rock was encountered at a depth of approximately 5.5 feet below existing grade.

Based on the hydraulic conductivity testing located within the footprint of the infiltration structure, D.L. Howell & Associates, Inc., recommends the following infiltration rate for the soils underlying Test 3-23-3: an infiltration rate of 1.68 inches per hour shall be used.

• Infiltration Test 3-23-4 was conducted at approximately \pm 5.0 feet below existing grade, which corresponds to an approximate infiltration elevation of 298.0. One deep test pit was excavated at this location to a depth of 7.0 feet below existing grade. During excavation, groundwater was encountered at a depth of approximately 7.0 feet below existing grade.

Based on the hydraulic conductivity testing located within the footprint of the infiltration structure, D.L. Howell & Associates, Inc., recommends the following infiltration rate for the soils underlying Test 3-23-4: an infiltration rate of 1.96 inches per hour shall be used.

• Infiltration Test 3-22-5 was conducted at approximately \pm 6.0 feet below existing grade, which corresponds to an approximate infiltration elevation of 334.0. One deep test pit was excavated at this location to a depth of 8.0 feet below existing grade. No limiting conditions were encountered at the time of excavation.

Based on the hydraulic conductivity testing located within the footprint of the infiltration structure, D.L. Howell & Associates, Inc., recommends the following infiltration rate for the soils underlying Test 3-22-5: an infiltration rate of 2.81 inches per hour shall be used.

• Infiltration Test 3-22-6 was conducted at approximately \pm 2.0 feet below existing grade, which corresponds to an approximate infiltration elevation of 306.0. One

deep test pit was excavated at this location to a depth of 6.0 feet below existing grade. No limiting conditions were encountered at the time of excavation.

Based on the hydraulic conductivity testing located within the footprint of the infiltration structure, D.L. Howell & Associates, Inc., recommends the following infiltration rate for the soils underlying Test 3-22-6: an infiltration rate of 2.93 inches per hour shall be used.

• Infiltration Test 3-22-7 was conducted at approximately \pm 2.0 feet below existing grade, which corresponds to an approximate infiltration elevation of 336.0. One deep test pit was excavated at this location to a depth of 4.0 feet below existing grade. During excavation, rock was encountered at a depth of approximately 4.0 feet below existing grade.

Based on the hydraulic conductivity testing located within the footprint of the infiltration structure, D.L. Howell & Associates, Inc., recommends the following infiltration rate for the soils underlying Test 3-22-7: an infiltration rate of 0.88 inches per hour shall be used.

• Infiltration Test 3-22-8 was conducted at approximately \pm 2.0 feet below existing grade, which corresponds to an approximate infiltration elevation of 314.0. One deep test pit was excavated at this location to a depth of 4.0 feet below existing grade. During excavation, rock was encountered at a depth of approximately 4.0 feet below existing grade.

Based on the hydraulic conductivity testing located within the footprint of the infiltration structure, D.L. Howell & Associates, Inc., recommends the following infiltration rate for the soils underlying Test 3-22-8: an infiltration rate of 0.43 inches per hour shall be used.

Please reference plan drawings for exact locations and visual representation of infiltration tests and test pits. Results of the hydraulic conductivity testing and soil horizon descriptions can be found in the enclosed attachments.

Hydraulic Conductivity Calculation

Coefficient of Permeability: $K = [A/(F^*D^*t)] \times \ln(h1/h2)$

- Where: K = permeability (inches per hour) A = cross sectional area of cased hole
 - F = shape factor (2.75 constant of flat bottom)
 - D = cased hole diameter
 - t = time for head change from h1 to h2
 - h1 = initial height of water column in casing
 - h2 = final height of water column in casing

*Reference *Soil Hydraulic Conductivity Analysis Form* for infiltration testing data and *Soil Morphology Form* for soil profile data.



Stormwater Infiltration Testing &

Hydraulic Conductivity Calculations

3/22/2021

DD

DATE:

BY:

JOB NO.: <u>3868</u> LOCATION: <u>1013 Shiloh Road</u> MUNICIPALITY: <u>Westtown Township, Chester County, Pa.</u> DESCRIPTION: <u>Stormwater Infiltration Testing</u>

Field Test Results

WEATHER CONDITIONS: <u>SUNNY</u> PRECIPITATION IN LAST 24 HOURS:

TEMPERATURE: 62 °F

<u>None</u>

			Readings								
Hole #	Depth (Inches)	Pre-Soak	1st	2nd	3rd	4th	5th	6th	7th	8th	
Test 3-22-5	72										
Time(min.)		30	30	30	30	30	30	30	30	30	
Drop(inches)		15.00	14.75	14.75	14.50	14.50	n/a	n/a	n/a	n/a	
Initial Water Leve	el Depth (inches)	18	18	18	18	18	n/a	n/a	n/a	n/a	

			Readings								
Hole #	Depth (Inches)	Pre-Soak	1st	2nd	3rd	4th	5th	6th	7th	8th	
Test 3-22-6	24										
Time(min.)		30	30	30	30	30	30	30	30	30	
Drop(inches)		15.25	15.00	14.75	14.75	14.75	n/a	n/a	n/a	n/a	
Initial Water Leve	Initial Water Level Depth (inches)		18	18	18	18	n/a	n/a	n/a	n/a	

Determination of Hydraulic Conductivity (Kv)

Kv = [A/(F*D*t)] * ln(h1/h2)

Kv = Vertical Permeability

- A = Cross-sectional area of cased hole
- F = shape factor (2.75 constant for flat bottom)
- D = cased hole diameter
- t = time for head to change from h1 to h2
- h1 = initial height of water column in casing
- h2 = final height of water column in casing

Test 3-22-5 Results

2.8062	(in/hour)
7.06858	(Sq.in.)
2 75	(Units)

-	()
3	(Inches)
0.5	(hrs.)
18	(Inches)

18 (Inches) 3.50 (Inches)

Test 3-22-6 Results

2.93319	(in/hour)
7.06858	(Sq.in.)
2.75	(Units)
3	(Inches)
0.5	(hrs.)
18	(Inches)
3.25	(Inches)



Stormwater Infiltration Testing &

Hydraulic Conductivity Calculations

3/22/2021

DD

DATE:

BY:

JOB NO.: <u>3868</u> LOCATION: <u>1013 Shiloh Road</u> MUNICIPALITY: <u>Westtown Township, Chester County, Pa.</u> DESCRIPTION: <u>Stormwater Infiltration Testing</u>

Field Test Results

WEATHER CONDITIONS: <u>SUNNY</u> PRECIPITATION IN LAST 24 HOURS:

TEMPERATURE: 62 °F

<u>None</u>

			Readings								
Hole #	Depth (Inches)	Pre-Soak	1st	2nd	3rd	4th	5th	6th	7th	8th	
Test 3-22-7	24										
Time(min.)		30	30	30	30	30	30	30	30	30	
Drop(inches)		7.25	7.25	7.25	7.25	7.25	n/a	n/a	n/a	n/a	
Initial Water Leve	el Depth (inches)	18	18	18	18	18	n/a	n/a	n/a	n/a	

			Readings								
Hole #	Depth (Inches)	Pre-Soak	1st	2nd	3rd	4th	5th	6th	7th	8th	
Test 3-22-8	24										
Time(min.)		30	30	30	30	30	30	30	30	30	
Drop(inches)		4.25	4.00	4.00	4.00	4.00	n/a	n/a	n/a	n/a	
Initial Water Leve	Initial Water Level Depth (inches)		18	18	18	18	n/a	n/a	n/a	n/a	

Determination of Hydraulic Conductivity (Kv)

$Kv = [A/(F^*D^*t)]^* ln(h1/h2)$

Kv = Vertical Permeability

- A = Cross-sectional area of cased hole
- F = shape factor (2.75 constant for flat bottom)
- D = cased hole diameter
- t = time for head to change from h1 to h2
- h1 = initial height of water column in casing
- h2 = final height of water column in casing

Test 3-22-7 Results

	-
0.8833	(in/hour)
	/ -

 7.06858
 (Sq.in.)

 2.75
 (Units)

 3
 (Inches)

 0.5
 (hrs.)

18 (Inches) 10.75 (Inches)

Test 3-22-8 Results

0.43065	(in/hour)
7.06858	(Sq.in.)
2.75	(Units)
3	(Inches)
0.5	(hrs.)
18	(Inches)
14.00	(Inches)

7.0685 2.75 3 0.5



Stormwater Infiltration Testing &

Hydraulic Conductivity Calculations

3/23/2021

DD

DATE:

BY:

JOB NO.: 3868 LOCATION: 1013 Shiloh Road MUNICIPALITY: Westtown Township, Chester County, Pa. **DESCRIPTION:** Stormwater Infiltration Testing

Field Test Results

WEATHER CONDITIONS: SUNNY **PRECIPITATION IN LAST 24 HOURS:**

TEMPERATURE: 64 °F

<u>None</u>

			Readings								
Hole #	Depth (Inches)	Pre-Soak	1st	2nd	3rd	4th	5th	6th	7th	8th	
Test 3-23-1	60										
Time(min.)		30	30	30	30	30	30	30	30	30	
Drop(inches)		11.75	10.50	10.50	10.50	10.50	n/a	n/a	n/a	n/a	
Initial Water Leve	el Depth (inches)	18	18	18	18	18	n/a	n/a	n/a	n/a	

			Readings									
Hole #	Depth (Inches)	Pre-Soak	1st	2nd	3rd	4th	5th	6th	7th	8th		
Test 3-23-2	48											
Time(min.)		30	30	30	30	30	30	30	30	30		
Drop(inches)		10.50	8.75	8.75	8.75	8.75	n/a	n/a	n/a	n/a		
Initial Water Level Depth (inches)		18	18	18	18	18	n/a	n/a	n/a	n/a		

Determination of Hydraulic Conductivity (Kv)

Kv = [A/(F*D*t)] * ln(h1/h2)

Kv = Vertical Permeability

- A = Cross-sectional area of cased hole
- F = shape factor (2.75 constant for flat bottom)
- D = cased hole diameter
- t = time for head to change from h1 to h2
- h1 = initial height of water column in casing
- h2 = final height of water column in casing

Test 3-23-1 Results

I	1.5002	(in/hour)
	7.06858	(Sq.in.)
	2.75	(Units)

7.50

3 (Inches) 0.5 (hrs.) 18 (Inches)

(Inches)

1.14082 (in/hour) 7.06858 (Sq.in.) 2.75 (Units) 3 (Inches) 0.5 (hrs.) 18 (Inches) (Inches) 9.25

Test 3-23-2 Results



Stormwater Infiltration Testing &

Hydraulic Conductivity Calculations

JOB NO.: 3868 LOCATION: 1013 Shiloh Road MUNICIPALITY: Westtown Township, Chester County, Pa. DESCRIPTION: Stormwater Infiltration Testing

Field Test Results

WEATHER CONDITIONS: SUNNY **PRECIPITATION IN LAST 24 HOURS:**

TEMPERATURE: 64 °F

<u>None</u>

			Readings										
Hole #	Depth (Inches)	Pre-Soak	1st	2nd	3rd	4th	5th	6th	7th	8th			
Test 3-23-3	42												
Time(min.)		30	30	30	30	30	30	30	30	30			
Drop(inches)		11.75	11.25	11.25	11.25	11.25	n/a	n/a	n/a	n/a			
Initial Water Leve	l Depth (inches)	18	18	18	18	18	n/a	n/a	n/a	n/a			

			Readings									
Hole #	Depth (Inches)	Pre-Soak	1st	2nd	3rd	4th	5th	6th	7th	8th		
Test 3-23-4	60											
Time(min.)		30	30	30	30	30	30	30	30	30		
Drop(inches)		12.25	12.25	12.25	12.25	12.25	n/a	n/a	n/a	n/a		
Initial Water Leve	el Depth (inches)	18	18	18	18	18	n/a	n/a	n/a	n/a		

Determination of Hydraulic Conductivity (Kv)

Kv = [A/(F*D*t)] * ln(h1/h2)

Kv = Vertical Permeability

- A = Cross-sectional area of cased hole
- F = shape factor (2.75 constant for flat bottom)
- D = cased hole diameter
- t = time for head to change from h1 to h2
- h1 = initial height of water column in casing
- h2 = final height of water column in casing

Test 3-23-3 Results

1.68075	(in/hour)
	• •

7.06858 (Sq.in.) 2.75 (Units) 3 (Inches) 0.5 (hrs.) 18 (Inches)

6.75 (Inches)

Test 3-23-4 Results

1.95551	(in/hour)
7.06858	(Sq.in.)
2.75	(Units)
3	(Inches)
0.5	(hrs.)
18	(Inches)
5.75	(Inches)

DATE: 3/23/2021 DD

BY:



PIT NUMBER: TP 3-23-	1 DLH NUMBER:	3868	INVESTIGATOR	: DWD
DATE: <u>3/23/2021</u>	1 STATE: PA		COUNTY:	CHESTER
MUNICIPALITY:	WESTTOWN TOWN	SHIP	CLIENT: KEY	STONE CUSTOM HOMES
SUBDIVISION:	STOKES ESTATI	E	SITE LOCATION	I: 1013 SHILOH ROAD
MORPHOLOGIC DETER	MINATION: SEW	AGE ST	ORMWATER	SHWT SOILS

Horizon	Dep	oth	Bound	dary	Color Tex		oxturo	%CFs	F	REDC	X	Structure	Consistence	NOTES	
HUHZUH	Upper	Lower	Distrnct	Торо	000	1	exture	70CFS	Α	S	С	Siluciule	Consistence	NOTES	
	0	11	А	W	10 YR 4/2	SIL	T LOAM	0				GRAN	FRI		
	11	46	А	W	10 YR 5/6	SIL	TY CLAY	0				MA	FIRM		
	46	84			VAR	SAN	NDY SILT	0				GRAN	LO		
COMMEN grade.	TS: This D	eep Test	Pit was c	onducte	ed at Test 3-23-1.	During ex	cavation, rock w	as enco	unter	red at	a de	pth of appro	oximately 84 ind	ches below exist	ting
SOIL TYP	E:				Soil Drainage Class: Soil					Soil Scient	tist Signature:				
LIMITING	CONDITIC	N: Rock			Excessively Drai	ned	Somewhat Po	orly Draii	ned						
Туре: V	Vater R	ock N	<i>l</i> ottling		Well Drained		Poorly Draine	d							
Depth: ~84	4"				Moderately Well	Drained	Very Poorly D	Drained							
	WEATHER: 64° Sunny					МЕТІ	HOD:	Ex	cavator						
			SLOPE:				EXCAVATION DEPTH				PTH:		84"		
			COVER:		Meadow		LANDSCA			OSIT	ION:		SW		

REDOX – Redoxymorphic features (Drainage Mottling) A/S/C – Abundance/Size/Contrast

 $Roots/Pores-f-few,\,c-common,\,m-many\,/\,f-fine,\,m-medium,\,c-coarse$

1250 Wrights Lane West Chester, PA 19380



PIT NUMB	ER: <u>TP 3-23-</u> 2	2 DLH NUN	DLH NUMBER: 3868			TOR:	DWD
DATE:	3/23/2021	STATE: PA			COUNTY:		CHESTER
MUNICIPALITY:		WESTTOWN TOWNSHIP			CLIENT:	KEYS	TONE CUSTOM HOMES
SUBDIVISIO	SUBDIVISION:		ESTATE		SITE LOCATION		1013 SHILOH ROAD
MORPHOLO	MORPHOLOGIC DETERM		SEWAGE	ST	ORMWATER		SHWT SOILS

Horizon	Dep	oth	Boun	dary	Color	olor Texture		%CFs	F	REDC	X	Structure	Consistence	NOTES	
HUHZUH	Upper	Lower	Distrnct	Торо	COIOI	I	exture	%CFS	Α	S	С	Structure	Consistence	NOTES	
	0	9	А	W	10 YR 4/2	SIL	T LOAM	0				GRAN	FRI		
	9	35	А	IR	10 YR 5/6	SIL	TY CLAY	0				MA	FIRM		
	35	72			VAR	SAN	NDY SILT	0				GRAN	FRI		
COMMEN	TS: This D	eep Test	Pit was c	onducte	ed at Test 3-23-2.	No limitin	g conditions we	re identif	ied a	t the	time	of excavati	on.		
SOIL TYP	E:				Soil Drainage Cl	ass:						Soil Scient	tist Signature:		
LIMITING	CONDITIC	N:			Excessively Drai	ned	Somewhat Po	orly Drair	ned						
Туре: V	Vater R	ock N	lottling		Well Drained		Poorly Draine	d							
Depth: +72	2"				Moderately Well	Moderately Well Drained Very Poorly Drained									
	WEATHER: 64° Sunny METHO						HOD:	Ex	cavator						
			SLOPE:				EXCAVATION DEPTH						72"		
			COVER:		Meadow	dowLANDSCAPE POSITIO				ION:		S			

REDOX – Redoxymorphic features (Drainage Mottling) A/S/C – Abundance/Size/Contrast Roots/Pores – f – few, c – common, m – many / f – fine, m – medium, c – coarse

1250 Wrights Lane West Chester, PA 19380



PIT NUMBE	R: <u>TP 3-23-3</u>	DLH NUME	DLH NUMBER: 3868			TOR:	DWD
DATE:	3/23/2021	STATE:	PA	CC	OUNTY:		CHESTER
MUNICIPALITY:		WESTTOWN TOWNSHIP			IENT:	KEYS	TONE CUSTOM HOMES
SUBDIVISIO	SUBDIVISION:		STATE	SI	SITE LOCATION:		1013 SHILOH ROAD
MORPHOLOGIC DETERM		INATION:	SEWAGE	STORM	WATER		SHWT SOILS

Horizon	on Depth Boundary Color 7		exture	%CFs	F	REDC	X	Structure	Consistence	NOTES					
HUHZUH	Upper	Lower	Distrnct	Торо	000		exture	%CF5	Α	S	С	Siluciule	Consistence	NOTES	
	0	11	А	W	10 YR 4/2	SIL	T LOAM	0				GRAN	FRI		
	11	47	А	W	10 YR 5/6	SIL	TY CLAY	0				MA	FIRM		
	47	66			10 YR 3/4	STO	ONY SILT	<20				GRAN	LO		
COMMEN grade.	TS: This D	eep Test	Pit was c	onducte	ed at Test 3-23-1.	During ex	cavation, rock v	vas enco	unter	ed at	a de	pth of appr	oximately 66 ind	ches below exist	ting
SOIL TYP	E:				Soil Drainage Class:					Soil Scient	tist Signature:				
LIMITING	CONDITIC	N: Rock			Excessively Drai	ned	Somewhat Po	orly Drai	ned						
Type: V	Vater R	ock N	<i>l</i> ottling		Well Drained		Poorly Draine	d							
Depth: ~66	6"				Moderately Well	Drained	Very Poorly [Drained							
		W	EATHER:		64° Sunny					METI	HOD:	Ex	cavator		
			SLOPE:				EXCAVATION DEPTH				PTH:		66"		
			COVER:		Meadow		LA	ANDSCA	PE P	OSIT	ION:		S		

REDOX – Redoxymorphic features (Drainage Mottling) A/S/C – Abundance/Size/Contrast Roots/Pores – f – few, c – common, m – many / f – fine, m – medium, c – coarse

1250 Wrights Lane West Chester, PA 19380



PIT NUMBER:	TP 3-23-4	DLH NUMBER:	3868	INVESTIGATOR	R: DWD
DATE: 3/2	23/2021	STATE:	PA	COUNTY:	CHESTER
MUNICIPALITY:	N	ESTTOWN TOWN	ISHIP	CLIENT: KEY	STONE CUSTOM HOMES
SUBDIVISION:		STOKES ESTAT	E	SITE LOCATIO	N: 1013 SHILOH ROAD
MORPHOLOGI		IATION: SEV	AGE ST	ORMWATER	SHWT SOILS

Horizon	Dep	oth	Boun	dary	Color	г	Toxturo	%CFs	F	REDC	X	Structure	Consistence	NOTES	
Horizon	Upper	Lower	Distrnct	Торо	Color	I	Fexture	%CFS	Α	S	С	Structure	Consistence	NOTES	
	0	8	А	W	10 YR 4/2	SIL	T LOAM	0				GRAN	FRI		
	8	31	А	W	10 YR 4/4	SIL	TY CLAY	0				MA	FIRM		
	31	47	G	W	10 YR 6/4	SIL	T LOAM	0				MA	FRI		
	47	84			VAR	SAN	NDY SILT					GRAN	LO		
COMMEN existing gr		eep Test	Pit was c	onducte	ed at Test 3-23-4.	During ex	cavation, ground	dwater w	as er	ncour	ntereo	l at a depth	of approximate	ely 84 inches bel	low
SOIL TYP					Soil Drainage Class:						Soil Scien	tist Signature:			
LIMITING	CONDITIC	N: Grou	ndwater		Excessively Drai	ined	Somewhat Po	orly Draii	ned						
Туре: И	Vater R	ock N	Nottling		Well Drained		Poorly Draine	d							
Depth: ~84	4"				Moderately Well	Drained	Very Poorly D	Prained							
-		W	EATHER:		64° Sunny					METI	HOD:	Ex	cavator		
			SLOPE:				EXCAVATION DEPTH						84"		
			COVER:		Meadow		LA	NDSCA	PE P	OSIT	ION:		S		

REDOX – Redoxymorphic features (Drainage Mottling) A/S/C – Abundance/Size/Contrast Roots/Pores – f – few, c – common, m – many / f – fine, m – medium, c – coarse

1250 Wrights Lane West Chester, PA 19380



PIT NUMBER: TP 3-22-	1 DLH NUMBER:	3868	INVESTIGATOR	E DWD
DATE: <u>3/22/2021</u>	STATE:	PA	COUNTY:	CHESTER
MUNICIPALITY:	WESTTOWN TOWNS	SHIP	CLIENT: KEY	STONE CUSTOM HOMES
SUBDIVISION:	STOKES ESTATE		SITE LOCATION	I: 1013 SHILOH ROAD
MORPHOLOGIC DETER	MINATION: SEW	AGE ST	ORMWATER	SHWT SOILS

Horizon	Dep	oth	Boun	dary	Color	-	Fexture	%CFs	F	REDC	X	Structure	Consistence	NOTES	
попдоп	Upper	Lower	Distrnct	Торо	000		exture	%CFS	Α	S	С	Structure	Consistence	NOTES	
	0	5	А	W	10 YR 4/2	SIL	T LOAM	0				SBK	FRI		
	5	50	А	W	7.5 YR 4/3	SIL	TY CLAY	0				MA	FIRM		
	50	96			VAR	SIL	T LOAM	0				GRAN	FRI		
COMMEN	TS: This D	eep Test	Pit was c	onducte	ed at Test 3-22-5.	No limitir	ng conditions we	re identif	ied a	t the	time	of excavati	on.		
SOIL TYPI	E:				Soil Drainage Cl	ass:						Soil Scient	tist Signature:		
LIMITING	CONDITIC	N:			Excessively Drai	ned	Somewhat Po	orly Draiı	ned						
Туре: V	Vater R	ock N	lottling		Well Drained		Poorly Draine	ed							
Depth: +96	6"				Moderately Well	Drained	Very Poorly D	Drained							
	WEATHER: 62° Sunny METHO								HOD:	Ex	cavator				
			SLOPE:			EXCAVATION DEPTH							96"		
			COVER:		Meadow	Meadow LANDSCAPE POSITION							SW		

REDOX – Redoxymorphic features (Drainage Mottling) A/S/C – Abundance/Size/Contrast Roots/Pores – f – few, c – common, m – many / f – fine, m – medium, c – coarse

1250 Wrights Lane West Chester, PA 19380



PIT NUMBER: TP 3-22	-6 DLH NUMBER:	3868	INVESTIGATOR	C DWD
DATE: <u>3/22/2021</u>	STATE:	PA	COUNTY:	CHESTER
MUNICIPALITY:	WESTTOWN TOWNS	SHIP	CLIENT: KEY	STONE CUSTOM HOMES
SUBDIVISION:	STOKES ESTATE		SITE LOCATION	N: 1013 SHILOH ROAD
MORPHOLOGIC DETER	MINATION: SEW	AGE ST	ORMWATER	SHWT SOILS

Horizon	Dep	oth	Boun	dary	Color	-	Fexture	%CFs	F	REDC	X	Structure	Consistence	NOTES	
попдоп	Upper	Lower	Distrnct	Торо	COIOI		lexiure	%CFS	Α	S	С	Siluciule	Consistence	NOTES	
	0	7	А	W	10 YR 4/2	SII	_T LOAM	0				MA	FRI		
	7	35	А	W	10 YR 4/3	SIL	TY CLAY	0				MA	FIRM		
	35	72			VAR	SAI	NDY SILT	0				GRAN	FRI		
COMMEN ⁻	TS: This D	eep Test	Pit was c	onducte	ed at Test 3-22-6.	No limitin	g conditions wer	e identifi	ed at	the	time c	of excavation	on.		
SOIL TYPI	E:				Soil Drainage Cl	rainage Class: Soil Scientist Signature:									
LIMITING	CONDITIC	N: Rock			Excessively Drai	ined	Somewhat Po	orly Draii	ned						
Туре: V	Vater R	ock N	<i>l</i> ottling		Well Drained		Poorly Draine	ed							
Depth: ~84	4"				Moderately Well	Drained	Very Poorly D	Prained							
		W	EATHER:		62° Sunny	i				METI	HOD:	Ex	cavator		
			SLOPE:				EXCAVATION DEPTH						84"		
			COVER:		Meadow		LA	PE P	OSIT	ION:		SW			

REDOX – Redoxymorphic features (Drainage Mottling) A/S/C – Abundance/Size/Contrast Roots/Pores – f – few, c – common, m – many / f – fine, m – medium, c – coarse

1250 Wrights Lane West Chester, PA 19380



PIT NUMBER: TP 3-2	2-7 DLH NUMBER:	3868	INVESTIGATO	R: DWD
DATE: <u>3/22/2021</u>	STATE:	PA	COUNTY:	CHESTER
MUNICIPALITY:	WESTTOWN TOWNS	SHIP	CLIENT: KE	YSTONE CUSTOM HOMES
SUBDIVISION:	STOKES ESTATE		SITE LOCATIO	N: 1013 SHILOH ROAD
MORPHOLOGIC DETE	RMINATION: SEW	AGE ST	ORMWATER	SHWT SOILS

Horizon	De	pth	Boun	dary	Color	-	exture	%CFs	F	REDO	ЭХ	Structure	Consistence	NOTES	1
HOHZOH	Upper	Lower	Distrnct	Торо	COIOI		exture	%CFS	Α	S	С	Structure	Consistence	NOTES	
	0	4	А	W	10 YR 4/2	SIL	T LOAM	0				GRAN	FRI		
	4	48			10 YR 5/4	STO	ONY SILT	<20				GRAN	FRI		
COMMEN [®] grade.	TS: This D	eep Test	Pit was c	onducte	ed at Test 3-22-7.	During ex	cavation, rock	was enco	untei	red a	t a de	pth of appr	oximately 48 in	ches below exist	ling
SOIL TYP	E:				Soil Drainage Cl	ass:						Soil Scien	tist Signature:		
LIMITING	CONDITIC	DN: Rock			Excessively Drai	ned	Somewhat Po	oorly Drai	ned						
Type: V	Vater R	ock N	/lottling		Well Drained		Poorly Draine	d							
Depth: ~48	3"				Moderately Wel	I Drained	Very Poorly	Drained							
		W	EATHER:		62° Sunny					MET	HOD:	Ex	cavator		
			SLOPE:					EXCAVA		N DE	PTH:		48"		
			COVER:		Meadow		L	ANDSCA	PE P	osi	FION:		NE		

REDOX – Redoxymorphic features (Drainage Mottling) A/S/C – Abundance/Size/Contrast Roots/Pores – f – few, c – common, m – many / f – fine, m – medium, c – coarse

1250 Wrights Lane

West Chester, PA 19380



PIT NUMBER: TP 3	DLH NUMBER:	3868	INVESTIGATOR:	DWD
DATE: 3/22/202	21 STATE:	PA	COUNTY:	CHESTER
MUNICIPALITY:	WESTTOWN TOWNS	HIP	CLIENT: KEYS	STONE CUSTOM HOMES
SUBDIVISION:	STOKES ESTATE		SITE LOCATION	: 1013 SHILOH ROAD
MORPHOLOGIC DET	ERMINATION: SEW	AGE ST	ORMWATER	SHWT SOILS

Horizon	Dep	oth	Bound	dary	Color	т	exture	%CFs	F	REDC	X	Structure	Consistence	NOTES	
110112011	Upper	Lower	Distrnct	Торо	COIOI	I	exture	/0013	Α	S	С	Siluciule	Consistence	NOTES	
	0	3	А	W	10 YR 3/1	SIL	T LOAM	0				GRAN	FRI		
	3	11	А	W	2.5 Y 5/3	SIL	TY CLAY	0				MA	FRI		
	11	31	G	W	10 YR 5/6	SIL	TY CLAY	0				MA	FRI		
	31	48			10 YR 4/4	STO	ONY SILT	<20				GRAN	FRI		
COMMENTS: This Deep Test Pit was conducted at Test 3-22-8. During excavation, rock was encountered at a depth of approximately 48 inches be grade.											ches below exist	ting			
SOIL TYP	E:				Soil Drainage Class: Soil						Soil Scient	tist Signature:			
LIMITING	CONDITIC	N: Rock			Excessively Drai	ned	Somewhat Po	orly Draii	ned						
Type: V	Vater R	ock N	Nottling		Well Drained		Poorly Drained	b							
Depth: ~48	3"				Moderately Wel	I Drained	Very Poorly	Drained							
		W	EATHER:		62° Sunny					METI	HOD:	Ex	cavator		
			SLOPE:					EXCAVA		N DE	PTH:		48"		
			COVER:		Woodlands		LA	NDSCA	PE P	OSIT	ION:		NE		

REDOX – Redoxymorphic features (Drainage Mottling) A/S/C – Abundance/Size/Contrast

 $Roots/Pores-f-few,\,c-common,\,m-many\,/\,f-fine,\,m-medium,\,c-coarse$

1250 Wrights Lane West Chester, PA 19380

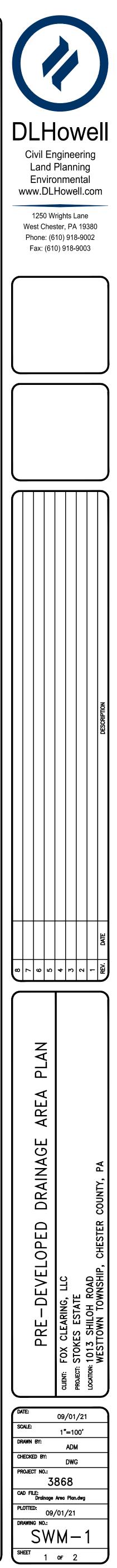


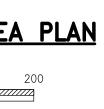
PRE-DEVELOPED DRAINAGE AREA PLAN SCALE: 1"=100'

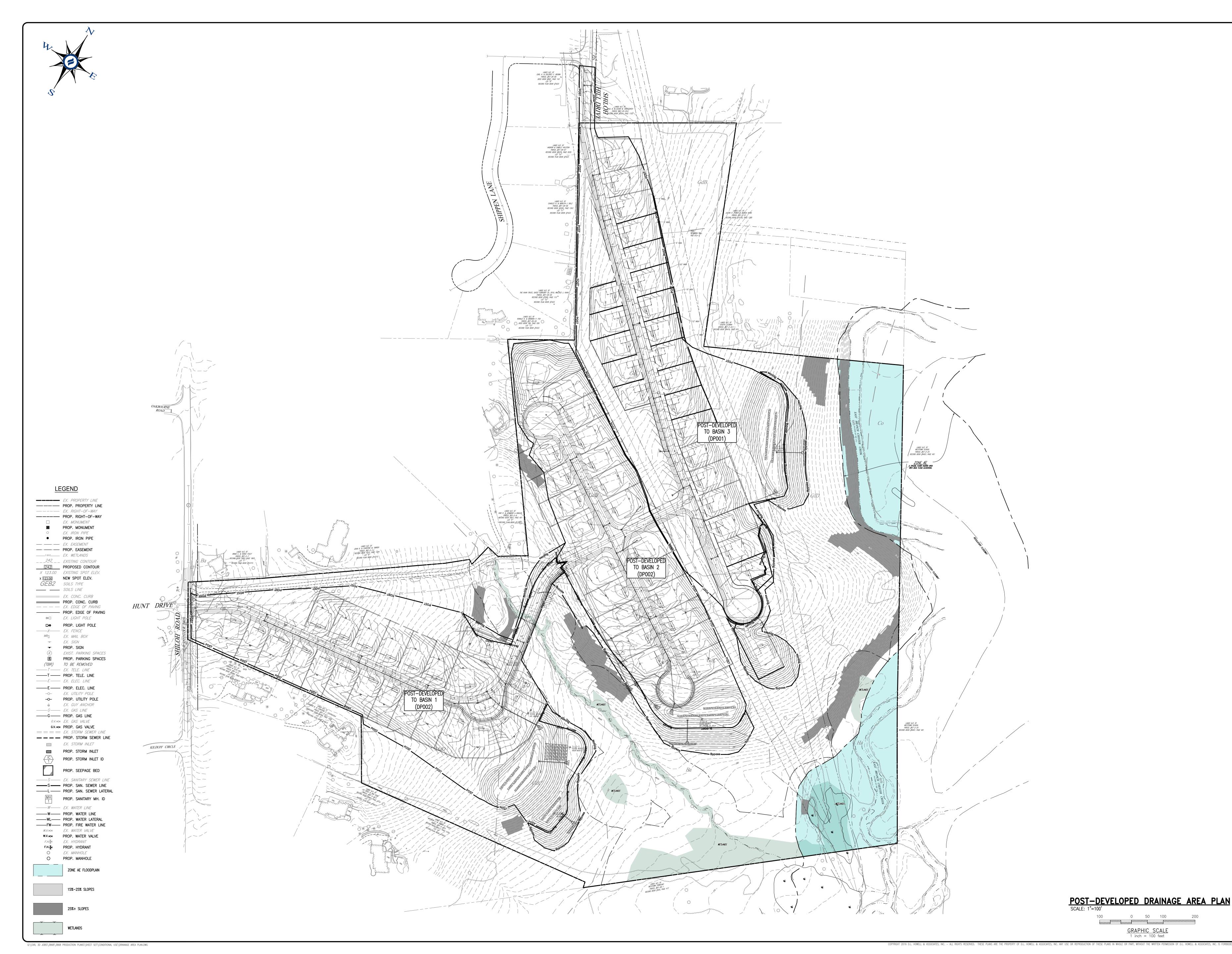
100

 $\frac{\text{GRAPHIC SCALE}}{1 \text{ inch } = 100 \text{ feet}}$

0 50 100





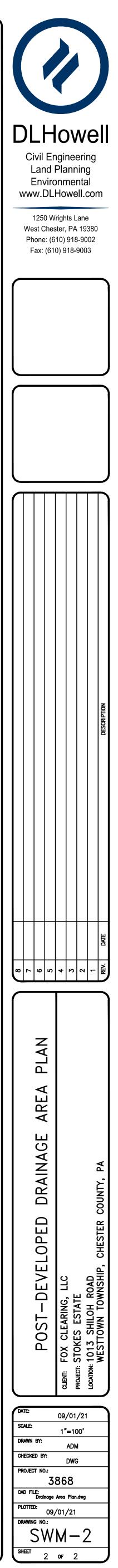


POST-DEVELOPED DRAINAGE AREA PLAN SCALE: 1"=100'

100

 $\frac{\text{GRAPHIC SCALE}}{1 \text{ inch } = 100 \text{ feet}}$

0 50 100



200